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ATTORNEYS AT LAW

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February 21, 2018

Planning Board of the City of Newburyport Attn: Bonnie Sontag, Chair Newburyport City Hali 60 Pleasant Street Newburyport, MA 01950

Re:

Modification of Approved Special Permit and Definitive Subdivision Plan at Donahue

Court

Dear Chair and Members of the Board:

This office is counsel to Stephen J. McConnell (the "Applicant"), who is the owner in his capacity as Trustee of the Stephen J. McConnell Revocable Trust of Lot 4B on Donahue Court (the "Property"). This letter constitutes a request by the Applicant for modification of a special permit and definitive subdivision plan concerning the Property, each of which has been previously approved by the Planning Board of the City of Newburyport (the "Board") and is discussed *infra*. Enclosed herewith, please find the following documents:

- "Special Permit Findings & Decision" on application by 13 North Adams LLC, approved May 20, 2015;
- 2. "Definitive Subdivision Plan Application and Decision Summary" on application by 13 North Adams LLC, approved May 20, 2015;
- 3. "Residential Development, Definitive Subdivision, Donahue Court (26 Toppans Lane Lot 4), Newburyport, Massachusetts, Prepared For: 13 North Adams LLC" prepared by Millennium Engineering, Inc., dated March 18, 2015 (the "Definitive Subdivision Plan");
- 4. "Application for Plan Examination and Building Permit Foundation Only" approved November 17, 2016 (the "Foundation-Only Permit");
- 5. December 1, 2016 Letter of Lisa A. Mead, Esq. to City of Newburyport Building Commissioner, without enclosures;

- 6. City of Newburyport Building Commissioner Stop Work Order Letter of December 8, 2016 (the "Stop Work Order");
- 7. City of Newburyport Department of Public Services Stormwater Permitting Violation Notice of December 20, 2016 (the "Stormwater Violation Notice");
- 8. "Stormwater Management Report for a Proposed Residential Subdivision, Donahue Land, Newburyport, Massachusetts" prepared by Design Consultants, Inc., revised through April 28, 2015 (the "Original Stormwater Report");
- 9. Stormwater Management Permit Application, approved February 8, 2017 (the "Stormwater Application");
- 10. February 12, 2018 Photographs;
- 11. Site Plan: "Plan of Land in Newburyport, MA Showing Proposed Single-Family Dwelling at Lot 4B Donahue Court" prepared by Millennium Engineering Inc., dated February 6, 2018 (the "Site Plan"); and
- 12. "Stormwater Calculations For: Steve McConnell, Proposed Single Family Dwelling, Lot 4B Donahue Court, Newburyport, MA" prepared by Millennium Engineering Inc., dated February 6, 2018 (the "Stormwater Calculations").

On May 20, 2015, the Board approved a special permit for the development of a private court for a proposed two lot residential subdivision, the private way of which proposes to serve the Property from the public Toppans Lane. On May 20, 2015, the Board approved a definitive subdivision plan for the development of a two lot residential subdivision consistent with the Definitive Subdivision Plan, which was later approved by the Newburyport Building Department on November 17, 2016.

In mid-November, 2016, the Applicant engaged Mark A. DePiero and his limited liability company DePiero, LLC (collectively, "DePiero) to perform the following site work at the Property in conformity with the approved special permit and definitive subdivision plan: removal of trees from the site, grading of the site, excavation for perimeter walls, and clearing of the site. On November 17, 2016, the Newburyport Building Department approved and issued the Foundation-Only Permit for the Property. Soon thereafter, DePiero immediately commenced site work at the Property.

On December 1, 2016, Lisa A. Mead, counsel to Virginia and Thomas Eramo (the "Eramos"), requested from the Newburyport Building Commissioner "appropriate enforcement action(s)" for the Property being developed purportedly in violation of the approved Definitive Subdivision Plan. The Eramos' property shares its northern boundary line with the Property. In support of her request, Attorney Mead alleged that the clear cutting and the cutting of significant grades, together with the removal of soils from the Property, near the shared boundary line created hazardous conditions for her clients and their property.

On December 8, 2016, the Newburyport Building Commissioner issued to the Applicant the Stop Work Order to cease all site work at the Property, because, according to the Building Commissioner, "the site work underway at the [Property] may not comply with the recorded site plan approved by the Newburyport Planning Board, dated 5/20/2015." Without waiving any rights under the existing approved special permit and definitive subdivision plan, the Applicant substantially complied with the Stop Work Order.

On December 20, 2016, the City of Newburyport Department of Public Services issued to the Applicant the Stormwater Violation Notice, which, *inter alia*, commanded that the Applicant cease all work at the Property until a permit has been issued and the conditions complied with. Without waiving any rights under the existing approved special permit and definitive subdivision plan, the Applicant substantially complied with the Stormwater Violation Notice. On February 7, 2017, the Department of Public Services of the City of Newburyport approved the Applicant's Stormwater Application.

On June 26 and 27, 2017, Gatchell & Sons Excavation Inc. ("Gatchell") performed excavation and remedial site work at the Property to address the concerns raised by the Eramos. Over the two days at the Property, Gatchell, which was monitored by Scott Frary of Country Road Landscaping, moved, graded, and compacted three hundred (300) yards of fill, two-hundred and forty (240) yards of which was brought in from off-site. The site work leveled and reinforced the area adjacent to the shared boundary, including, but not limited to, the Eramos' asphalt driveway and adjoining land, all as shown on the February 12, 2018 photographs enclosed herewith.

On January 25, 2018, in light of the active Stop Work Order at the Property and recent regrading of the Property by Gatchell, Alexander F. Parker, a Massachusetts Title V Certified Soil Evaluator, conducted soil testing at the Property. Following that testing, Christopher M. York, a Massachusetts registered civil engineer with Millennium Engineering, Inc., produced the Stormwater Calculations and the Site Plan submitted herewith. The Stormwater Calculations authored by Mr. York supplements and updates the Original Stormwater Report for the Property, which was previously approved by the Board.

The Site Plan modifies and improves on those plans included in the Definitive Subdivision Plan as follows: enlargement of the house footprint from approximately 2,230 square feet to 3,818 square feet; enlargement of the driveway area from 1,720 square feet to 3,440 square feet; inclusion of stone walls; enlargement of basketball court; inclusion of stone trench to infiltrate stormwater runoff from enlarged basketball court; enlargement in area of detention basin from 233 square feet to 440 square feet due to enlargement of house footprint and driveway area; and more detailed site grading. Prior to the March 21, 2018 Board hearing, the Applicant will submit to the Board a supplemental site plan for the Property showing, *inter alia*, his proposed tree implants and other screening mechanisms for the shared boundary line with the Eramos discussed *supra*.

For the reasons discussed above, the Applicant respectfully requests that the Board modify the previously approved special permit and definitive subdivision plan consistent with the Stormwater Calculations and Site Plan enclosed herewith.

This request does not constitute an acknowledgement or admission that the Applicant is in any way in breach of his obligations under the special permit and definitive subdivision plan or the Applicant's agreement with the propriety of the Stop Work Order or Stormwater Violation Notice. This request is made at the suggestion of Newburyport officials and the Applicant reserves all rights under the previously approved special permit and definitive subdivision plan.

Thank you for your attention.

Very truly yours,

William H Sheeben III

Enc.

cc: Stephen J. McConnell

Andrew R. Port, Planning Director Peter Binette, Building Commissioner planning@cityofnewburyport.com

EXHIBIT



CITY OF NEWBURYPORT PLANNING BOARD

60 Pleasant Street • P.O. Box 550 NEWBURYPORT, MA 01950 (978) 465-4400 • (978) 465-4452 (FAX)

& PLAN

SPECIAL PERMIT FINDINGS & DECISION

DECISION DATE:

5/20/2015

FIGINAL REFERENCE REQUESTED

APPLICATION DATE:

3/19/2015

FILE NO:

2015-SP-02

APPLICANT/OWNER:

13 North Adams LLC

APPLICANT ADDRESS:

9 Pasture Road, Bedford, NH 03110

SITE ADDRESS:

26 Toppans Lane

MAP/LOT:

39-40

BK/PAGE:

33791-202

ZONING DISTRICT:

 \mathbb{R}^2

SECTION:

XXIII Courts and Lanes

BRIEF DESCRIPTION:

allow a private Court for a two lot residential subdivision

NEWSPAPER NOTICE:

3/31/2015 and 4/7/2015

PUBLIC HEARING:

A public hearing was held on the above application on

4/15/2015 and continued to 5/6/15, 5/20/15

DECISION:

After the close of the public hearing the Planning Board, upon a motion made by member Doug Locy and seconded by Andrew Shapiro, the Board voted to APPROVE the

application for a Special Permit.

The motion having received the necessary two-thirds super majority vote of all the members of the Planning Board, in accordance with M.G.L. Chapter 40A Section 9, as amended, the petition for the Special Permit was therefore granted.

RECORD OF VOTE:

The following members of the Planning Board voted as follows with respect to the petition for a Special Permit

subject to the below-stated terms and conditions:

Jim McCarthy Bonnie Sontag

Absent

Susan Grolnic Noah Luskin

Yes Absent Leah McGavern Absent James Brugger Abstain Andrew Shapiro Yes

Don Walters Yeş Doug Locy

Yes

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. City Clerk aryport, Massachusetts

Page 1 of 6

PLANS AND MATERIALS INCORPORATED INTO DECISION

- 1. This Special Permit application is accompanied and augmented by the following plans, drawings and submittals:
 - Plan set entitled "Residential Development Definitive Subdivision Donahue Court (26
 Toppans Lane-Lot 4) Newburyport, Massachusetts prepared for: 13 North Adams, LLC,
 9 Pasture Road, Bedford, NH" dated March 18, 2015 and revised 4/28/15 consisting of
 pages T1, S1, S2, C1 through C4, and D1 through D3.
- 2. The plans and other submission material were reviewed by the Planning Board, its legal counsel and consulting engineer as follows:
 - "Stormwater Management Report for a Proposed Residential Subdivision Donahue Lane, Newburyport, Massachusetts" prepared by Design Consultants, Inc. dated March 18, 2015 and revised April 28, 2015.
 - Comments from the following City departments: Molly Ettenborough, Recycling and Energy Manager, dated 4/30/15; Bob Bracey, Health Director, dated 4/7/15; Julia Godtfredsen, Conservation Adminstrator, dated 3/30/15, Steve Bradbury, Fire Deputy, dated 3/30/15
 - "Technical Review #1" comments, dated 4/7/15 and email communication, dated 5/20/15 from reviewing engineer Phil Christiansen.
 - 3 Throughout its deliberations the Planning Board has been mindful of the statements of the applicants and their representatives, and the comments of the general public, all as made at the public hearing.

FINDINGS

General:

This project involves a total of 1.28 acres, which will be subdivided into two lots, Lot 4A and Lot 4B, with 0.46 acres and 0.82 acres, respectively. The lots will meet all of the dimensional standards set forth in Section VI-A of the Newburyport Zoning Ordinance. An existing single-family home shall be demolished and replaced with a new, single-family home. The project will add only one, additional single-family residence to the neighborhood.

A new roadway, Donahue Court, will be constructed to provide common access to the two lots in the subdivision. It shall be constructed consistent with the conditions and waivers granted in this Board's Definitive Subdivision Plan Approval dated 5/26/15.

Special Permit Criteria:

Section XXIII of the Newburyport Zoning Ordinance states that the Planning Board may grant a Special Permit for a Court or a Lane to:

 enhance the public safety through reduced number and frequency of vehicle entry points to ways used by the public, particularly arterial streets;

- preserve, protect and enhance environmentally sensitive land; and
- encourage the protection and preservation of significant features.

Per Section X-H.7 and X-H.8, before granting an application for a Special Permit, the Board, with due regard to the nature and condition of all adjacent structures and uses, and the district within which the same is located, shall find all of the following general conditions to be fulfilled:

- 1. The use requested is listed in the table of use regulations or elsewhere as in the ordinances requiring a special permit in the district for which application is made or is similar in character to permitted uses in a particular district but is not specifically mentioned.
- 2. The requested use is essential and/or desirable to the public convenience or welfare.
- 3. The requested use will not create undue traffic congestion, or unduly impair pedestrian safety.
- 4. The requested use will not overload any public water, drainage or sewer system or any other municipal system to such an extent that the requested use or any developed use in the immediate area or in any other area of the city will be unduly subjected to hazards affecting health, safety or the general welfare.
- 5. Any special regulations for the use, set forth in the special permit table are fulfilled.
- 5. The requested use will not impair the integrity or character of the district or adjoining districts, nor be detrimental to the health or welfare.
- 7. The requested use will not, by its addition to a neighborhood, cause an excess of that particular use that could be detrimental to the character of said neighborhood.
- 8. The proposed use is in harmony with the purpose and intent of this ordinance.
- 9. The proposed use shall not be conducted in a manner so as to emit any dangerous, noxious, injurious or otherwise objectionable fire, explosion, radioactive or other hazard, noise or vibration, smoke, dust, odor or other form of environmental pollution.

Specific Findings:

- Donahue Court will not be used to satisfy zoning frontage requirements except as provided by the Definitive Subdivision Plan Approval dated 5/26/15.
- The court shall serve two (2) single-family dwelling units/lots.

DECISION

In view of the foregoing, the Planning Board hereby decides to grant a XXIII Courts and Lanes Special Permit in accordance with the terms and conditions stated below:

General Conditions:

1. The applicant shall file this Special Permit Decision with the Southern Essex County Registry of Deeds or registry of the Land Court if registered land and a copy of the decision stamped with the recording information (Book/Page or Land Court document number) shall be included with the application for a Building Permit. The final site/construction plans shall also make reference to the decision date and conditions of approval. This Special Permit shall rum with the land and be binding on all future owners of the property.

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- 2. Prior to the granting of a Building Permit, the applicant shall submit the written certification of a registered Architect or Civil Engineer to the Building Commissioner that the project will be constructed in accordance with the approved plans and conditions.
- 3. No building permit shall be issued unless all public utilities have been reviewed and approved by the respective utility departments and that the proposed utilities will be installed in accordance with plans submitted with the application.
- 4. Record plans stamped by a professional engineer showing new construction, renovation or expansion shall be submitted to the Building Commissioner and the Planning Board. Said plans shall contain a certification, made by a registered architect or civil engineer, that what was constructed is consistent with approved plans and conditions set forth by the Planning Board as part of this Special Permit.
- 5. The Planning Board requires the developer to submit all drawings and plans in computer aided drafting (CAD) format. Specific file format shall be .dxf unless otherwise determined by the Office of Planning and Development.
- 6. All design and construction of all water utilities must meet Federal, State and local standards and the latest version of Newburyport Water Works Construction Guidelines and Construction Policies, which are available at the Water Division Business Office in City Hall. Contractors are prohibited from operating or connecting to any part of the existing water system without written authorization and field oversight by the Department of Public Services Water Division.
- 7. All design and construction of all sewer utilities must meet Federal, State and Local standards and the latest version of Newburyport Sewer Department Construction Guidelines and Construction Policies, which are available at the Wastewater Treatment Plant located at 115 Water Street. Contractors are prohibited from operating or connecting to any part of the existing sewer system without written authorization and field oversight by the Department of Public Services Sewer Division.
- 8. This Special Permit is valid for two years from the date from filing with the City Clerk. Special Conditions:
- 1. Prior to requesting building permits within the project, the applicant shall provide to the Planning Board (through the Office of Planning & Development) Homeowner's Association or Trust documents. Said documents shall ensure sufficient provisions are in place to ensure proper monitoring and implementation of the proposed "Operation and Maintenance Plan" for the proposed drainage system design. Proof of recording (for the final approved documents) shall be provided to the Planning Board (through the Office of Planning & Development) prior to requesting release of the performance guarantee.
- 2. The applicant, developer and/or successors in interest shall plow the strip of land along the edge of the road so that it is clear of snow during the winter season.
- 3. The home on Lot 4A shall be oriented toward Toppans Lane. There shall be no garage facing Toppans Lane.
- 4. The applicant shall install residential fire suppression systems in both hoes.
- 5. The structures on Lots 4A and 4B shall be single-family residences
- The applicant shall grant to the Newburyport Department of Public Services an easement over Donahue Court for the purposes of accessing the utilities in emergency situations.

DATE OF FILING OF DECISION

Filed with the Newburyport City Clerk on 5/26/2015 and sent registered mail to the applicant. Notice of decision is also sent by regular mail to the Parties in Interest and the abutting municipalities.

SIGNATURE OF THE BOARD

Jim McCarthy, Chair, Newburyport Planning Board

5/26/2015 Date

NOTICE OF APPELLATE RIGHTS

Appeals shall be made pursuant to M.G.L. Chapter 40A Section 17 and filed within twenty (20) days after the date of filing this decision in the Office of City Clerk.

CITY CLERK CERTIFICATION OF APPEAL PERIOD EXPIRATION

Suhan Bertonan	
Certification of the City Clerk: I,	, City Clerk of the City of
Newburyport, hereby certify pursuant to M.G.L. Chapter 40A	Section 17, that the decision for the
property known as: was filed in the Office of the City Clerk of	on 5/26/2015
Pursuant to M.G.L. Chapter 40A Section 11, this decision was and Development on 5/26/2015 and twenty days have elapsed appeal has been filed. Appeals shall be made pursuant to M.G.	after the decision was filed and no

within twenty (20) days after the date of filing of this decision in the Office of the City Clerk.

Newburyport City Clerk

MAY 0 4 2016

Date

EXHIBIT



CITY OF NEWBURYPORT PLANNING BOARD

60 PLEASANT STREET • P.O. BOX 550 NEWBURYPORT, MLA 01950 (978) 465-4400 • (978) 465-4452 (FAX)

DEFINITIVE SUBDIVISION PLAN APPLICATION AND DECISION SUMMARY

DECISION DATE:

5/20/2015

APPLICATION DATE:

03/19/2015

FILE NO:

2015-DEF-01

APPLICANT:

13 North Adams LLC

APPLICANT ADDRESS: 9 Pasture Road, Bedford, NH 03110

PARCEL(S) ADDRESS:

26 Toppans Lane

MAP/LOT:

39-40

BK/PAGE:

33791-202

ZONING DISTRICT:

R2

BRIEF DESCRIPTION: 2 lot residential subdivision

NEWSPAPER NOTICE: 03/31/2015 and 04/07/2015

PUBLIC HEARING:

A public hearing was held on the above application on 04/15/2015 and

continued to 5/20/2015.

DECISION:

After the close of the public hearing, upon a motion made by member Doug Locy and seconded by Don Walters, the Planning Board voted to

approve the definitive subdivision plan.

The motion having received a majority vote of all the members of the Planning Board, in accordance with M.G.L. Chapter 41 Section 81, as amended, the petition for the Definitive Subdivision Plan was therefore

APPROVED.

RECORD OF VOTE:

The following members of the Planning Board voted as follows with

respect to the perition for a Definitive Subdivision Plan approval to the

below-stated terms and conditions:

Jim McCarthy Don Walters

Yes

Leah McGavern Absent

Bonnie Sontag Absent

Absent

Sue Grolnic

Douglas Locy Yes

Noah Luskin James Brugger Abstain

Andrew Shapiro Yes

NUE COPY ATTEST

, City Clerk uryport, Massachusetts

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Page 1 of 6

PLANS AND MATERIALS INCORPORATED INTO DECISION

- 1) This Definitive Subdivision Application is accompanied and augmented by the following plans and drawings:
 - Plan set entitled "Residential Development Definitive Subdivision Donahue Court (26 Toppans Lane-Lot 4) Newburyport, Massachusetts prepared for: 13 North Adams, LLC, 9 Pasture Road, Bedford, NH" dated March 18, 2015 and revised 4/28/15 consisting of pages T1, S1, S2, C1 through C4, and D1 through D3.
- 2) Other submission materials include:
 - "Stormwater Management Report for a Proposed Residential Subdivision Donahue Lane, Newburyport, Massachusetts" prepared by Design Consultants, Inc. dated March 18, 2015 and revised April 28, 2015.
 - Comments from the following City departments: Molly Ettenborough, Recycling and Energy Manager, dated 4/30/15; Bob Bracey, Health Director, dated 4/7/15; Julia Godtfredsen, Conservation Adminstrator, dated 3/30/15, Steve Bradbury, Fire Deputy, dated 3/30/15
 - "Technical Review #1" comments, dated 4/7/15 and email communication, dated 4/20/15 from reviewing engineer Phil Christiansen.
- 3) Throughout its deliberations the Planning Board has been mindful of the statements of the applicants and their representatives, and the comments of the general public, all as made at the public hearing.

FINDINGS

Pursuant to Section 5.7 of the Rules and Regulations Governing the Subdivision of Land in Newburyport, the Board generally finds the following to be credible statements about the proposed Subdivision:

- 1. Complete and technically adequate plans and supporting material;
- 2. Adequate access to all of the lots in the subdivision by ways that will be safe and convenient for travel;
- 3. Lessened congestion in such ways and in the adjacent public ways;
- 4. Reduced danger to the life and limb in the operation of motor vehicles;
- 5. Secured safety in the case of fire, flood, panic and other emergencies;
- 6. Compliance with applicable zoning ordinances;
- 7. Adequate provisions for water, sewerage, drainage, underground utility services, fire, police, and other similar municipal equipment, and street lighting and other requirements where necessary in a subdivision;
- 8. Coordination of the ways in neighboring subdivisions;
- 9. Conformance with the design and construction standards described in the Rules and Regulations Governing the Subdivision of Land and in the attached Appendices; and
- 10. Conformance with all applicable zoning requirements.

WAIVERS

In accordance with M.G.L. Chapter 41 Section 81-R Waiver of Compliance and in consideration of the above referenced findings, the following waivers are granted as such actions are in the public interest and not inconsistent with the intent and purpose of the subdivision control law.

- Waiver of requirements of Sections 1.3.3 and 5.4.2(d): Vertical Datum
 Waiver is granted. The applicant shall utilize NAVD 1988 rather than NGVD 1929
- Waiver of requirements of Section 5.4.2(a): Plan Scale
 Waiver is granted. The plans shall have a scale of 1" = 20' rather than 1" = 40'
- Waiver of requirements of Section 5.6: Environmental and Community Impact Analysis Waiver is granted. The applicant shall not be required to submit (1) an Alternatives Analysis, (2) a Traffic Impacts Analysis, or (3) a Cost Benefit Analysis due to the negligible impact of one, additional single family home on City services and on the surrounding neighborhood.
- Waiver of requirements of Section 6.8.1: Minimum Design Standards for Courts
 Waiver is granted. The applicant shall construct a curb radius of 15' rather than the required 25' at the intersection of Donahue Court and Toppans Lane.
- Waiver of requirements of Section 6.9: Curbing and Section 6.11.1: Sidewalks
 Waiver is granted. Due to the relatively small size of this subdivision (one additional single-family home, for a total of two, single-family homes), there shall be no curbing or sidewalks installed along Donahue Court.

CONDITIONS

This Definitive Subdivision Plan approval is conditioned upon the following:

General Conditions:

- 1. After expiration of the appeal period yet before its endorsement of the plan, the Board shall require a performance guarantee pursuant to Section 5.8 to insure the construction of ways and the installation of municipal services.
- 2. In accordance with Section 5.9, the Definitive Plan must receive endorsement by a majority of Planning Board members and be stamped by the City Clerk. Endorsement follows expiration of the twenty (20) day appeal period.
- 3. Per Section 4 of the Planning Board's Regulations Governing Fees and Fee Schedules, Project Review Fees shall be paid in full prior to endorsement of the plan.
- 4. As per Section 5.9.2, the applicant/developer shall record this Definitive Plan approval, subject to its waivers and conditions, along with plans, covenants, and easements, at the Essex South Registry of Deeds and notify the Planning Board of such recording. If the endorsement on the plan or the certificate which accompanies the plan is not dated within six (6) months of the date of recording, the applicant shall apply to the Planning Board or City Clerk for a certificate which shall be endorsed on the plan or referred to on the plan and recorded with the plan. The certificate must be dated within thirty (30) days of the recording. The certificate shall state: "that the approval has not been modified, amended, or rescinded, nor the plan changed."

- 5. Section 5.10 requires that the applicant shall send by registered mail to the City Clerk and to the Planning Board a written statement that said construction or installation has been completed in accordance with the Rules and Regulations upon the completion of such construction and installation to serve any lot or lots. The statement shall include the address of the applicant.
- 6. Approval by the Board of a Definitive Subdivision Plan shall not constitute acceptance by the City of Newburyport of any street, sidewalk, or other municipal service within the subdivision as per Section 5.14.1.
- 7. Pursuant to Section 5.15, plan approval shall automatically lapse if the applicant/developer fails to complete construction of all ways and installation of municipal services in a subdivision within three (3) years of the date of approval.
- 8. The Board, its officers, and agents may enter upon any lands and there make examinations and surveys or to place and maintain monuments and marks as far as they deem necessary in carrying out the subdivision control law, pursuant to Section 5.16.
- 9. The developer shall provide the Planning Office and Department of Public Services with both hard copies and digital copies (AutoCAD and .pdf) of As-Built drawings upon completion of the project. Shop drawings shall be submitted to the Department of Public Services at least two weeks prior to construction to allow for proper departmental review.
- 10. Approval by the Planning Board of this Plan shall not be treated as, nor deemed to be, approval by the Board of Health for a permit for the construction and use on any lot. No building or structure shall be placed on any lot without the consent of the Board of Health, if required.
- 11. There shall be no construction other than that shown on approved plans or additions to any structures or any new use of a structure or land in the subdivision unless the Planning Board shall have reviewed and approved such change.
- 12. Developer shall take reasonable care not to disturb surrounding properties and property owners during construction. Construction work shall be limited to the hours between 7 a.m. and 6 p.m., unless a specific waiver is requested. The Planning Board and the City Marshal shall approve such waiver.
- 13. The installation of all drainage and water service and utilities shall meet all requirements of the Department of Public Services. The applicant shall request and obtain proper visual inspection from the Department of Public Services prior to backfill of any pipes, gates, catch basins, curbing, and other utility structures.
- 14. Prior to any construction, the applicant shall provide the Office of Planning & Development with one (1) copy of the entire approved plan set in both AutoCAD and .pdf formats.
- 15. Prior to any construction, the applicant shall provide to the Office of Planning & Development an estimated Construction Schedule and Contact List for the General Contractor and any emergency contacts during construction.
- 16. Before any lots are released for building purposes, the necessary drainage (including retention or detention basins) shall be installed, water mains and water services to lot lines shall be installed, rough and finished grading shall be at proper grades and the first or base course of bituminous concrete shall be installed, all to the satisfaction of the Planning Board and to its delegated inspection agent, the Department of Services.
- 17. Lots shall be released upon deposit of suitable financial security against the completion of the subdivision. Whenever the applicant seeks approval for the issuance of Lot Releases or

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whenever the applicants seek to alter the amount of financial security being held by the board, the applicant shall submit a written request to the Office of Planning & Development to initiate the requisite inspectional services.

Specific Conditions:

- 1. The proposed construction will require regular inspection by the Planning Board's consultant engineer as well as the Department of Public Services (DPS). Such inspections are necessary to ensure that the proposed project is constructed in accordance with the approved plans, to summarize and estimate the cost of remaining work, and to immediately address any issues which may arise during the construction of the project. The cost of such review and inspections shall be borne by the applicant. To cover the cost of these services the applicant shall replenish and maintain a Project Review Fee of \$2,500. Such funds shall be held by the Planning Board in an escrow account. Whenever notified that the funds in said escrow account have depleted to less than twenty-five (25) percent of the initial Project Review Fee, the applicant shall immediately thereupon deposit sufficient funds to return the account to the initial balance. The balance of said peer review account shall be replenished to the initial Project Review Fee prior to any construction on site. Upon completion of the project, any remaining funds shall be returned to the applicant.
- 2. The applicant shall grant to the Newburyport Department of Public Services an easement over Donahue Court for the purposes of accessing the utilities in emergency situations.

DATE OF FILING OF DECISION

Filed with the Newburyport City Clerk on 5/26/2015 and sent registered mail to the applicant.

SIGNATURE OF THE BOARD

Jim McCarthy, Chair, Newburyport Planning Board

5/26/2015 Date

NOTICE OF APPELLATE RIGHTS

Appeals shall be made pursuant to M.G.L. Chapter 41 Section 81BB and filed within twenty (20) days after the date of filing this decision in the Office of City Clerk.

<u>CITY CLERK CERTIFICATION</u>	
· · · · · · · · · · · · · · · · · · ·	, City Clerk of the City of Newburyport, nitive Subdivision Plan approval has been received and ceeived during the twenty (20) days after such receipt and
Richard Between	MAY 0'4 2016
City Clerk .	Date

EXHIBIT

Solution 3

PAPEOVE BOLIONG DEPT. OITY OF LIEWING PROPER

RESIDENTIAL DEVELOPMENT **DEFINITIVE SUBDIVISION** (26 TOPPANS LANE-LOT4) DONAHUE COURT

NEWBURYPORT, MASSACHUSETTS 13 NORTH ADAMS LLC 9 PASTURE ROAD PREPARED FOR:

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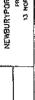
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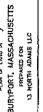








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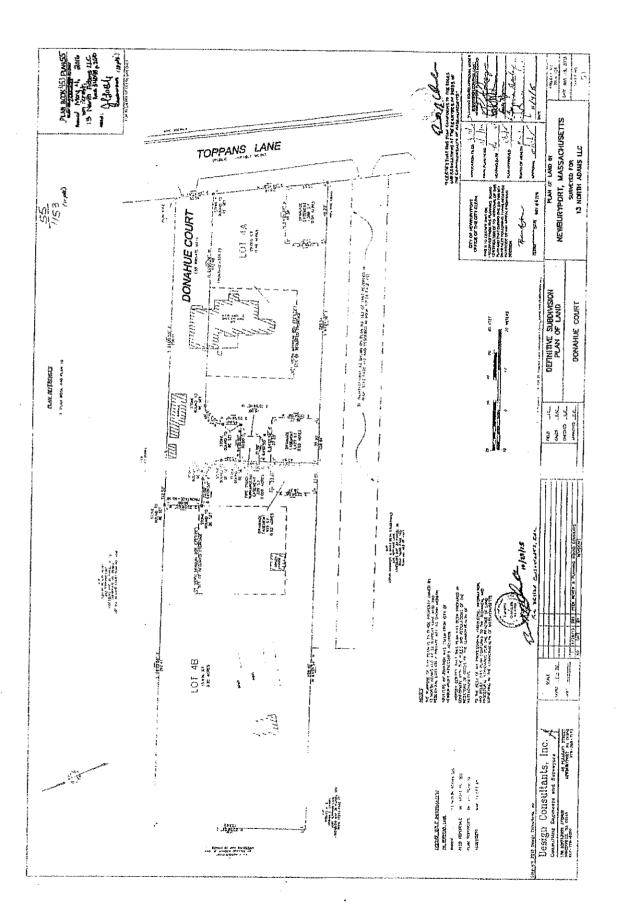
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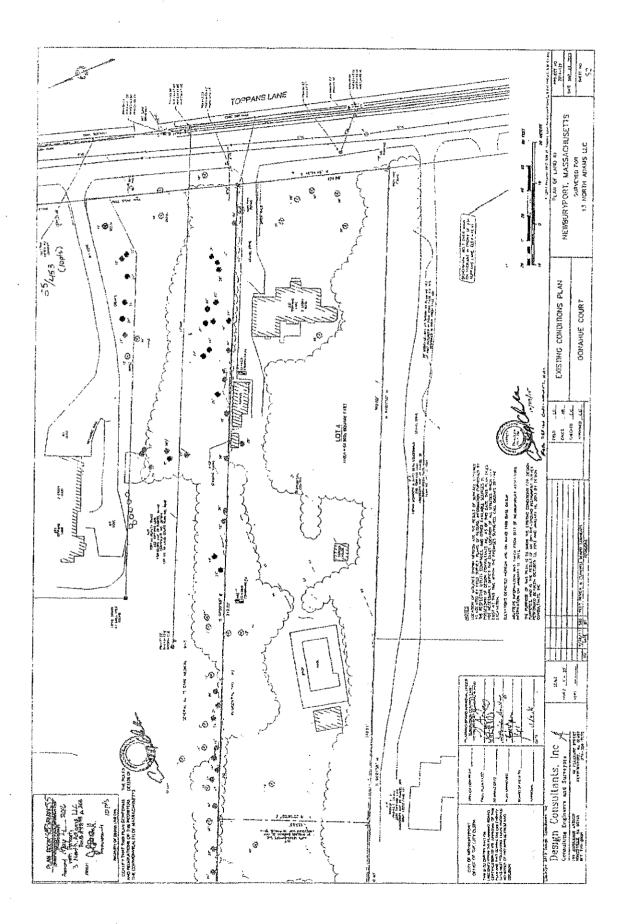
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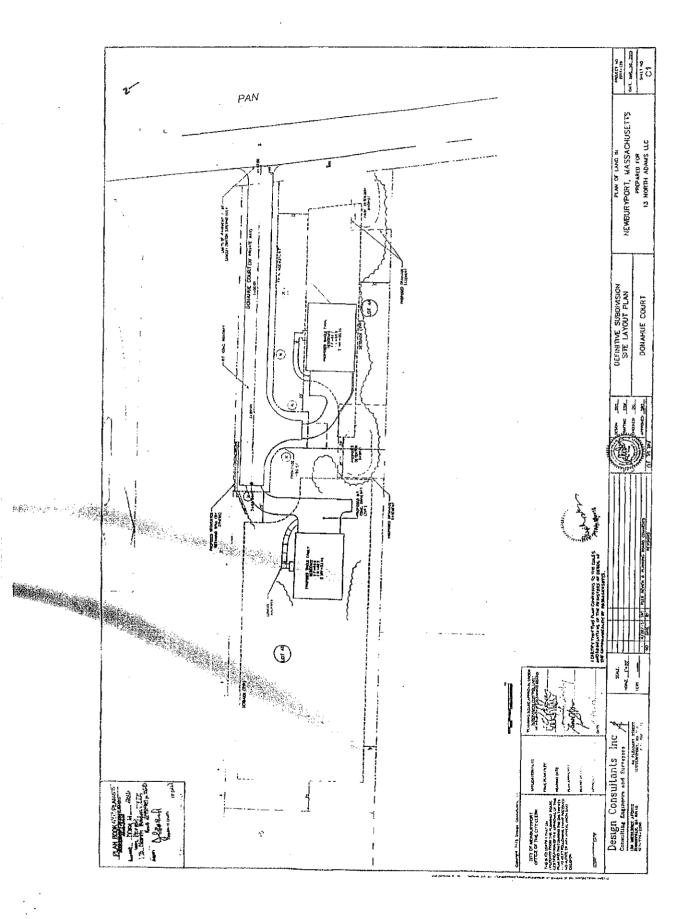
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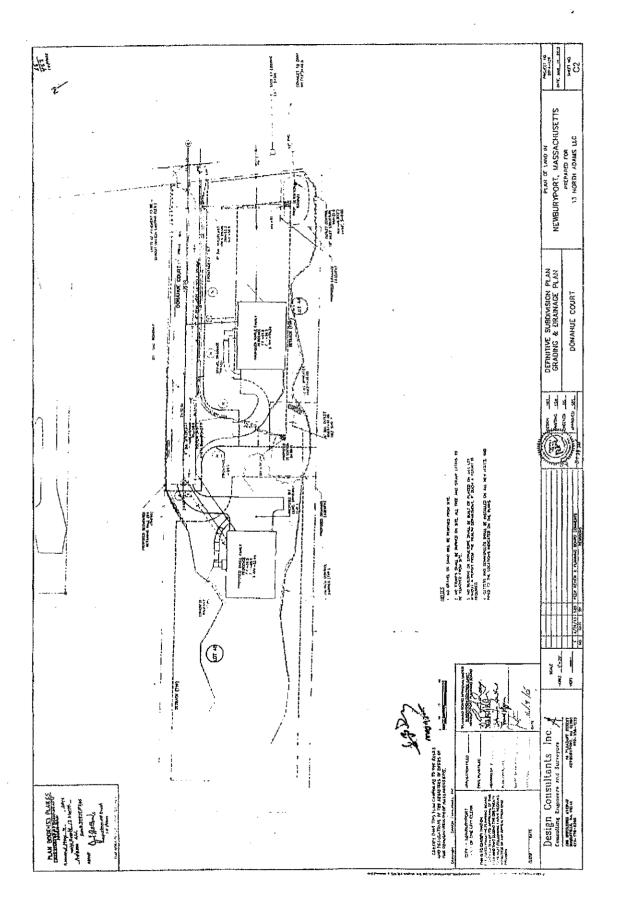
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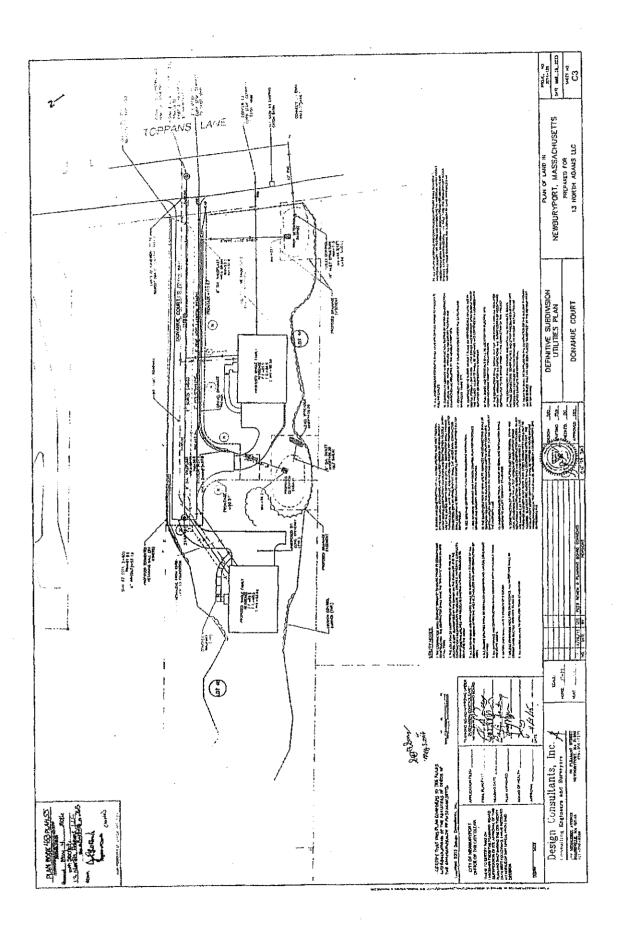
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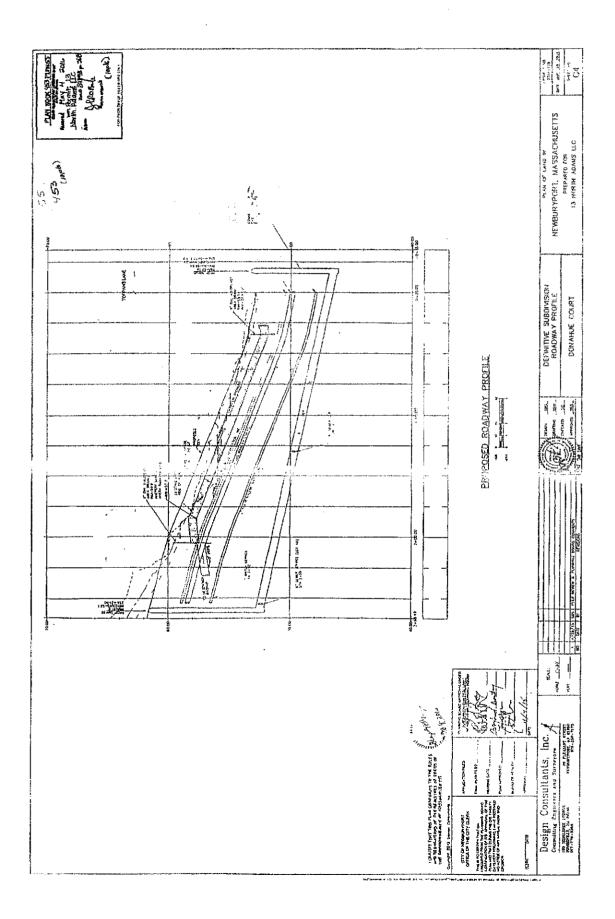


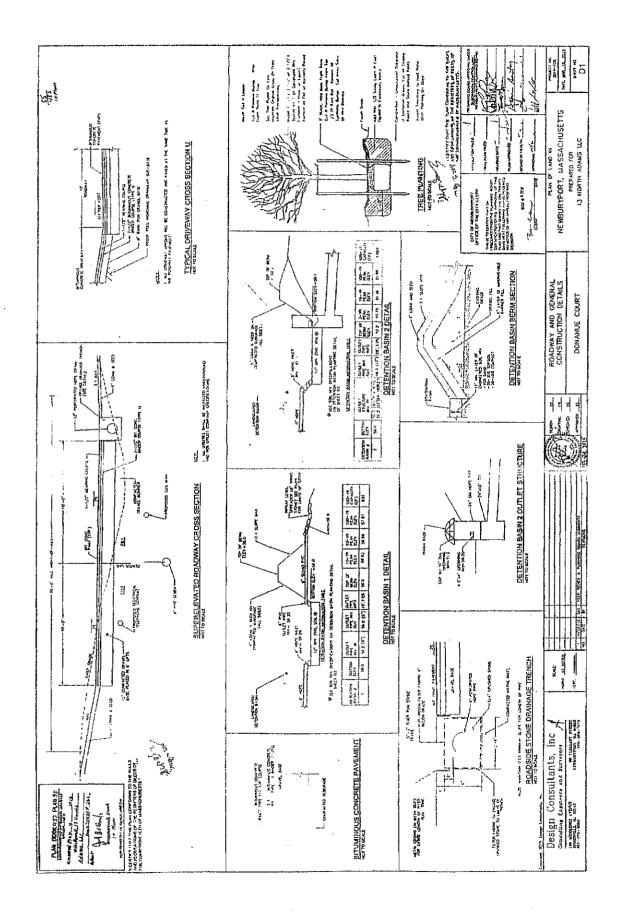


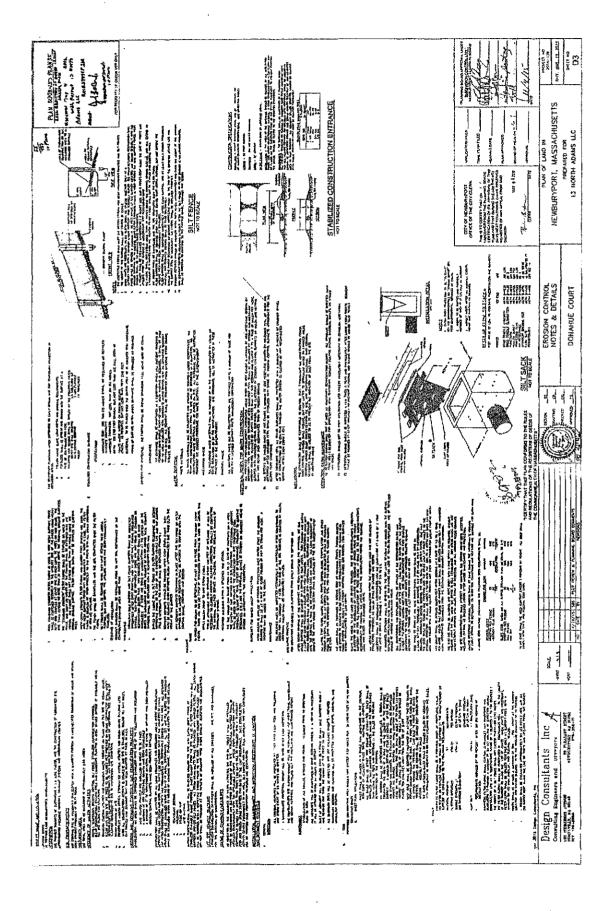


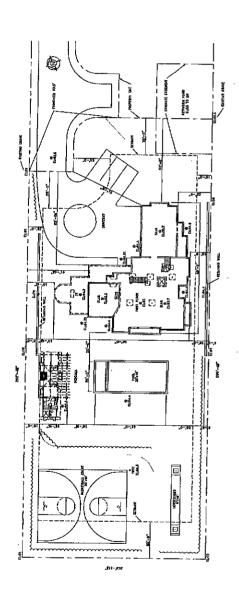






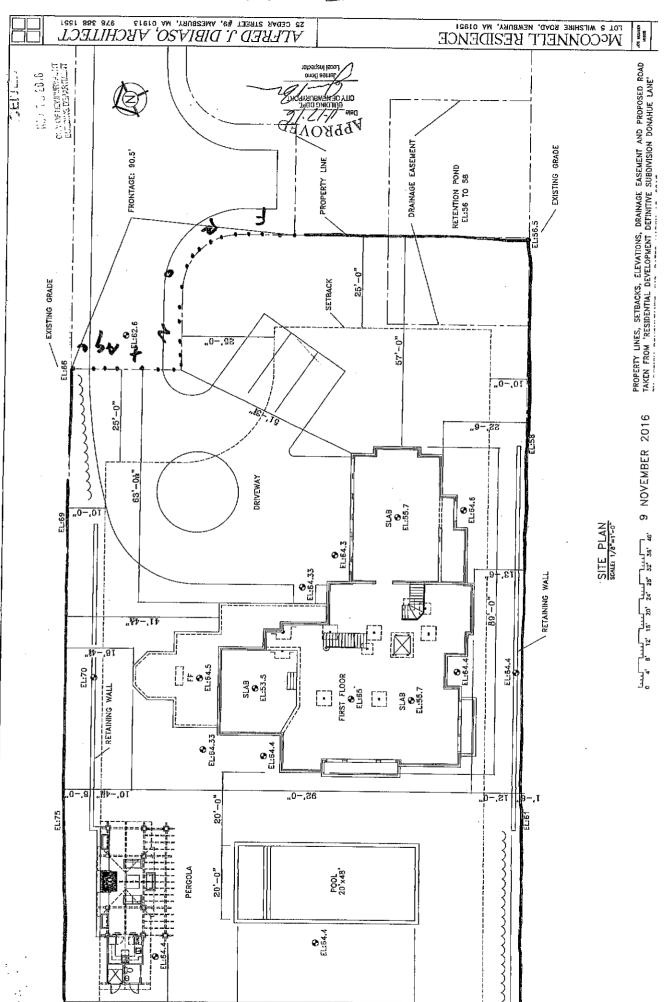






PROPERTY LINES, SETBACKS, ELEVATIONS, DRAINAGE EASEMENT AND PROPOSED ROAD TAKEN FROM 'RESIDENTIAL DEVELOPMENT DEFINITIVE SUBBIVISION DONAHUE LANE' 0 4' 8' 12' 16' 20' 34' 28' 32' 38' 40' 9 NOVEMBER 2016

1000 3714



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Brutatrus Structural

NEWBURY, MA 01951 LOT 5 WILSHINE ROAD MACONNELL RESIDENCE

1991 885 846 25 CEDAR STREET #9, AMESBURY, MA 01915

1. All jumber pressure treated Southern Pine No. 2; All connections galvanized or stainless steel, provide positive connection at all framing members example joint hangers or Simpson H2.5A or ii. Handenils by Ovener and Contractor iii. The General Contractors shall provide a continuous load path from upper level posts/columns/ framing to the foundation. If platform framing is utilized, block as required

Concrete, [Foundation Walls and Footings, Stebs, Piers, Big Foots, Etc.] Minimum 4,000 psi fo at 28 days reinforcing steel shall be deformed bars conforming to ASTM A615 Grade 60, fy = 60,000 psi

Ifferioral Bendencing Bars in the foundation walls and footings Shall be Confinuous At Conners/Steword Angles/Terninal Ends with Matching Conner Dowels Lap splice length 30 bar diameters U. N. O., Bend/Hook Down or Up at Terminal Ends 14" thick Minimum Concrete 4,000 past To at 28 days with 1.5 # Jenibe yard fiber meeh in mix over 6 mil Polyeth. Vapor Barrier, Typical a provide Control Joints 25 ft. o.c. and at re-artumt conners within 8 hours of pour a provide Control Joints 25 ft. o.c. and at re-artumt conners within 8 hours of pour hours 1 of Conners, # o.c. Remainder 1.2" Diameter 12" long 8"embodinent 4" projection with nut and washer at top and botton hours 1 of Conners, # o.c. Remainder 1.2" Diameter 12" long 8"embodinent 4" projection with nut and washer at top and botton 30 PACETILL AGAINST ONN TALLE UNITE. WALLS UNITE THE FIRST FLOOR FRAMING IS IN PLACE WITH SHEATHING AND BASE SLAB IS IN PLACE OR UNITE. WALLS ARE BRACED TO WITHERAND THE 38SURES OF BACKFILLING OPERATIONS.

nnet / Contractor shall Verity in Field Soils capable to support 4,000 pat soil bearing pressure on shallow spread from thee draining soil concess suggest a Gore-sectional Figures the Consulted or construction of these beingtons that additional rainforcing uses detail upon the consequence of three beingtons shall be one vertical to three beingtons with additional rainforcing uses detail in the consequence of all Ladge and note a mixe of cach. If Ledge encountered remove for minimum one foot of compacted granular fill above ledge in 6" titls itenally. Remove soil down to ledge, thus all ledge bearing and backfill with 100 ps Loa Cooretto mix above Ledge.

ssign Criveria : Pauliding Code RCC 2009 vvitu Massachusetta Residential Code 8th Edition Addendum Wind 100 mpi Exposure C

Pg 50 psf Ground Snow, Soil Conditions assumed free draining soil capable to support 4,000 psf shallow foundation system, Contractor & Owner to Verify in Field, if there are concerns consult with a Geo-technical Engineer for direction & Contact Engineering to notify

ructural Steel (U.N.O. on drawings):

is recommended the General Confractor (GC) and steel supplier shall field verify dimensions and conditions after foundations, walls, etc, are placed but before steel is cut to length, punched, coped, drilled, etc. At its steel supplier and steel detailing professional to verify in field site dimensions, one if it field dimensions, etc. Field dimensional changes may affect beam fit upshooting alignments, etc. Beam details such as over the homing, etc. affect fit up and febrication dimensions.

(Fy 50 ksi): Plates, Channels, Angles (Fy 46 ksi). ASTM A500 Grade C Structural Tubing (Tube Steel = TS or HSS)

(Fy 36 km)

ASTM A36

(steel to steel connections) A325N :: ii. (snebor belge) (A307 or A356) :: RAM valls (hollow block) or into massury brick walls (consistent belge) and sold connected by the consistent provide dilled 2 part spoxy resin, Hill Hir HY-20 verial screen label it into CM/Umassory well cavities where defilled 2 part spoxy resin, Hill Hir HY-20 with screen label it into CM/Umassory well cavities are proved edited 2 part spoxy resin, Hill Hir HY-20 with screen label it into CM/Umassory well cavities and calculated 2 part spoxy resin, Hill Hir HY-20 with screen label it into extent practical tuniess noted otherwise (U.C.N.) on the design drawings. All field connections shall be belowed to the extent practical tuniess noted otherwise (U.C.N.) on the design drawings. Accordinate with cleanies and field connections in the wests sendor all magnetic and vertex in the area of the partial state of the other of the design, fabrication, and evertion of structural steel for building of the AlSC.

rify in Field (V.I.F) all dimensions and conditions, underground utilities, "DIG-SAFE" coordination, etc.

rdinite tress General Notes with General Notes on all drawings. In cases of discrepancy, General Notes on this sheet govern for Structural issues, and Notes are part of this design document. The Central Contractor (GC) and Owner shell review and coordinate appropriately, thous and thosan so Construction is the responsibility of the General Contractor (GC). Temporary showing, if required, is the responsibility of the General Contractor (GC). Temporary showing, if required, is the responsibility of the GC. spearant Contractor (GC) are amended by the Massachusetts Residential State Building Code for this design is the IRC 2009 as amended by the Massachusetts Residential State Building Code 8th Edition [MSBC] Addendum Bearral Contractor (GC) shell:

then all work per the governing building code above and Local Codes
ordinates Students Drawings with all drawing, all stop drawings if applicable, all trades, etc.
Discoppancies are anothered (In Dimensional or Physical Conditions) noify the Engineer Phor to proceeding with that Portion of the work
deemed Contractor stall provide a continuous load peth from upper level posts'scolumns/framing to the foundation. If platform framing is utilized, block as required between floor levels, etc. Do not use "Platform Framing" at Great Rooms with no ceiling at wall plates, that is, open to above, use full height continuous well stude
so in the existing soil strats is entiable for shallow spread footings ans shallow strip footings capable to support 4,000 psf soil bearing pressure. If concerns hire and coordinate with a Geo-technical Engineer

andward, Estences, connectors, nails, seriews, etc. used with Pressure Treated Limber (P.T.) shall be, as recommended by the Fressure Treated Wood Industry for use with treated wood:
andward, Estences, connectors, nails, seriews, etc. used with Pressure Treated Limber (P.T.) shall be, as recommended by the Fressure Treated Wood Industry for use with the frestenes, they are considered dissimilar materials
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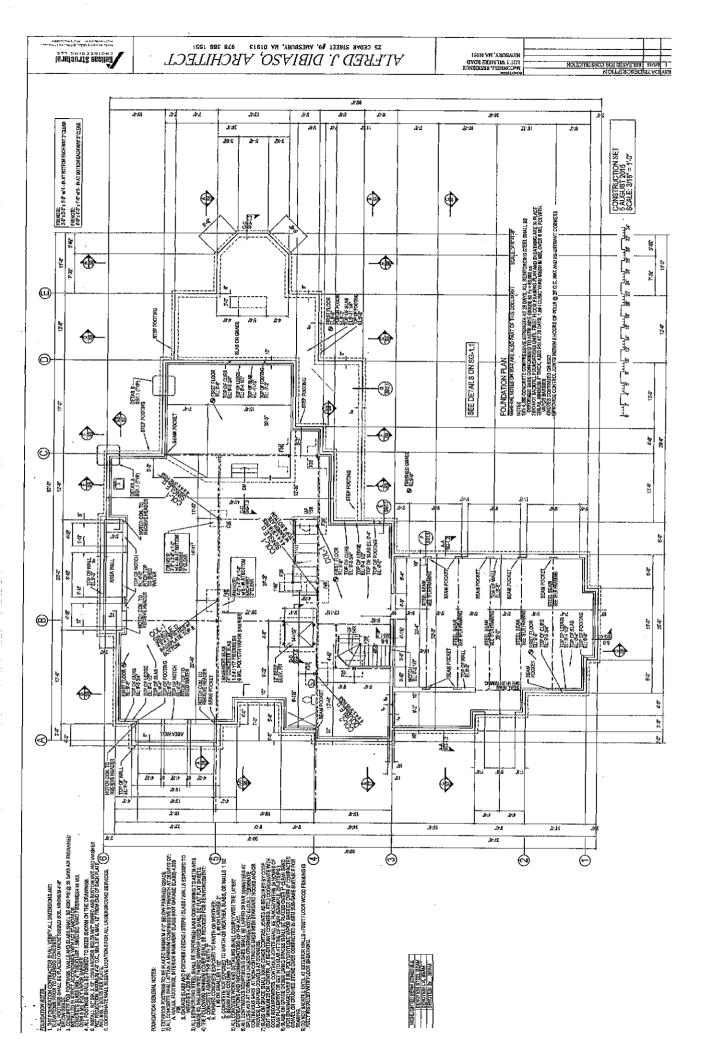
 PSL Columns/Posts (PSL [Parallel Strand Lumber]): i. Wolmunized (pressure treated

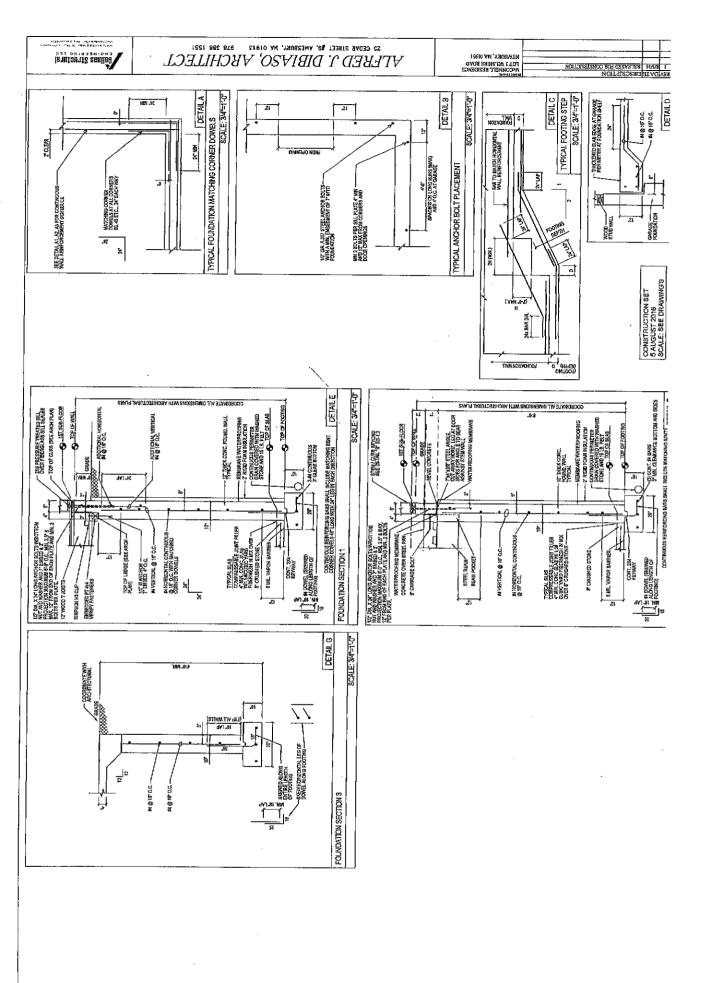
a. B = 1,566,000 psi; Fb = 1,728 psi; Fo|| = 1,450 psi
2. Beaus (Parallams, Service Level 2. Wolmanizedyw
a. B = 1,60,000 psi; Fe = 1,000 psi; Fv = 1,750 psi
a. B = 1,000 psi; Fv = 1,000 psi

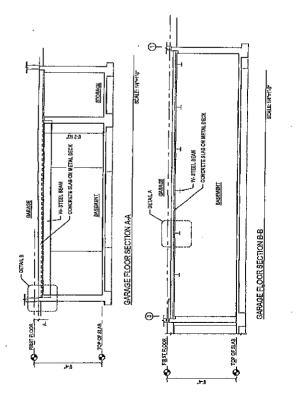
undations / Soil Bearing Additional Notes:

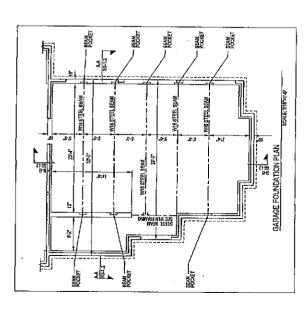
iteel Shop Drawings submittal for raview prior to steel flabrication is recommended ted materials shall conform to the following (minimum):
Structural W Shapes (Fv 50 k)

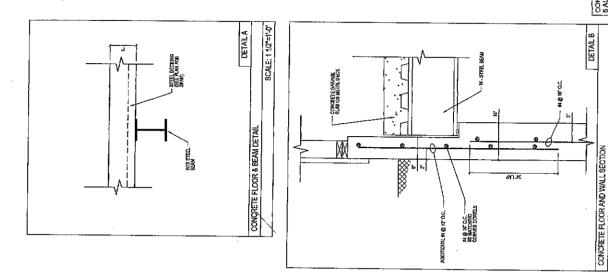
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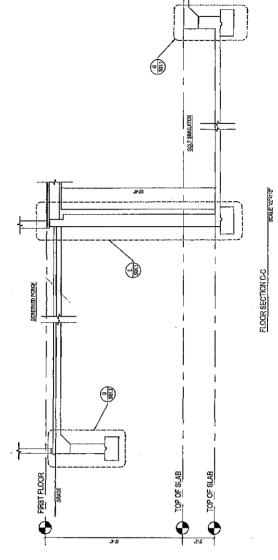


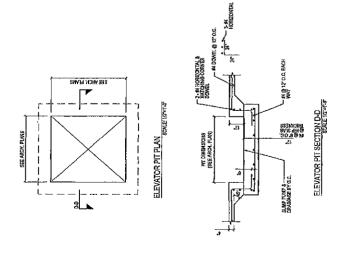












EXHIBIT



eter Binette uilding Commissioner/Codes Administrator

CITY OF NEWBURYPORT

BUILDING DEPARTMENT

P.O. Box 550

NEWBURYPORT, MA 01950 978-465-4405

ason 978-423-8502 26 Toppens Jane

Permit No: 3-51

Zoning District:

APPLICATION FOR PLAN EXAMINATION AND BUILDING PERMIT

uilding Permit...

THE MUNICIPAL BUILDING CODE prohibits the erection, conversion, repair, alteration, or enlargement of any building or structure - or the installation, alteration, or enlargement of any of the mechanical service equipment (electrical, plumbing, heating, gas systems) without first obtaining the necessary and required permits.

uilding Plans...

THE MUNICIPAL BUILDING CODE requires that application for permits be accompanied by at least two (2) complete sets of dimensioned plans and specification showing all necessary details to enable the BUILDING OFFICIAL to determine if the proposed work will be in compliance with the code.

uilding Permit Fees...

SCHEDULE OF PERMIT FEES:

\$50,00 for the first \$1,000.00 of the cost estimate.

\$10.00 for each additional \$1,000.00 of the cost estimate or part thereof.

uilding Inspections...

THE MUNICIPAL BUILDING CODE requires one (1) copy of the approved plans and specifications together with the work or building permit to be kept at the site/work area of operation at all times during progress of the work as authorized by the Building Commissioner. It is the of the person obtaining the permit to notify the Building Commissioner when the work is ready for inspection and no work should be covered before it has been inspected and approved. REQUESTS FOR INSPECTION are usually required in accordance with the following schedule:

1st -- Footings after forms are set and prior to placement of concrete materials.

2nd - Foundation walls prior to backfilling.

3rd – Rough framing prior to application of insulation or interior wall coverings. All service equipment (electrical, plumbing, heating, and gas systems rough-in) prior to cover or concealment.

4th – Final inspection by the Building Commissioner prior to a permitted use and occupancy.

5th - And on such special occasions as the Building Commissioner may designate.

local Inspector

NOV 15 2016

RECEIVED

CITY OF NEWBURYPORT BUILDING DEPARTMENT

		# # ph 2 2 2		Service .
ADBRESS		* * 3.4	ZC	DNING DISTRICT
BETWEEN	(cross street)	AND	10	(cross street)
SUBDIVISION		MAP	LOT <u>*</u>	LOT SIZE *
ST OF BUILDING -	- All applicant	s complete p	arts A - D)
ntial, enter number units added, if any in above) ent family residential, if units in building in & III. & III & Jon) al, corporation, State, or local	Residential 2 \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	y re family units notel, motel units pecify Describe in de shop, school, d at industrial par If USE of exi	Non- 18 [19 [20 [21 [22 [23 [24 [25 [26 [27 [28] 29] 29]	Residential Amusement, recreational Church, other religious Industrial Parking garage Service station, repair garage Hospital, institutional Office, bank, professional Public utility School, library, other educational Stores, mercantile Tanks, towers
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CHARACTERISTIC	CS OF BUILDI	ers, skip to VI.		s and additions, complete parts E
bearing) el ncrete y	40 Public 41 Privat H. TYPE OF 42 Public 43 Privat L I. TYPE OF	or private comp e (septic tank, e WATER SUPPL c or private comp te (well, cistern)	pany tc.) Y pany	48 Number of stories 49 Total square feet of floor area, all floors based on exterior dimensions 50 Total land area, s.f. K. NUMBER OF OFF STREET PARK SPACES 51- Enclosed 52 Outdoors L. RESIDENTIAL BUILDINGS ONLY
	SUBDIVISION ST OF BUILDING /EMENT Intial, enter number units added, if any in above) ent ifamily residential, funits in building in all. & III. &	SUBDIVISION ST OF BUILDING — All applicant /EMENT Intial, enter number units added, if any in above) ent ifamily residential, funits in building in the state, or local At the not included in the state of the st	SUBDIVISION	SUBDIMISION



CITY OF NEWBURYPORT BUILDING DEPARTMENT P.O. Box 550 NEWBURYPORT, MA 01950 978-465-4405

Peter Binette Building Commissioner/Codes Administrator

certain exceptions, along with other requirements.

AFFIDAVIT
Home Improvement Contractor Law
Supplement to Permit Application

MGL c. 142A requires that the "reconstruction, alteration, renovation, repair, modernization, conversion, improvement, removal, demolition, or construction of an addition to any pre-existing owner-occupied building containing at least one but not more than four dwelling units... or to structures which are adjacent to such residence or building be done by registered contractors, with

Type of Work:	Foundation only	Est. Cost: 27,300
Address of Work:	Υ	
Owner Name:	* Stephen J McCornell	
C of Permit Applica	tion: 11-14-2016	
Lhereby certify that:		
Registration is	not required for the following reason(s): Work excluded by law Job under \$1,000 Building not owner-occupied Owner pulling own permit other (specify):	
Notice is hereby giver	that:	
	THEIR OWN PERMIT OR DEALING WITH UNREGIST RK DO NOT HAVE ACCESS TO THE ARBITRATION P	
Signed under penaltie	es of perjury:	
I hereby apply for a.p	ermit as the agent of the owner:	•
Date	Contractor Name	Registration No.
OR		
. withstanding the a	above notice, I hereby apply for a permit as the owner of	the above property:

Pursuant to Chapter 9 of the Code of Ordinances for the City of any permit or license, the applicant must obtain sign-offs from the all local taxes, fees, assessments, betterments, or other municipal delinquent:	following Departments indicating that
Treasurer/Collector:	Date: // - /5 -/ &
DPS Sewer Division:	Date: 11-15-16
DPS Water Division:	Date: 11-15-16
26 709005	Foundation
REQUIRED: Pursuant to the Newburyport Zoning Ordinance, S for Building Permits must be include a list of all decisions o Commission, or Department relevant to this permit application, the Planning Board, Conservation Commission, Historical Confollowing permits, decisions, or approvals have been issued relational.	r permits issued by any City official, Board, This includes decisions and/or approvals from numission and Zoning Board of Appeals. The
Board/Department & Permit/Decision Type: Approval D	ate: File # (if applicable)
	,



The Commonwealth of Massachusetts Department of Industrial Accidents Office of Investigations 1 Congress Street, Suite 100 Boston, MA 02114-2017

www.mass.gov/dia

Workers' Compensation Insurance Affidavit: Builders/Contractors/Electricians/Plumbers Applicant Information Please Print Legibly Name (Business/Organization/Individual): 01833 City/State/Zip: (errnetous 9)8-912,-1783 Phone #: Are you an employer? Check the appropriate box: Type of project (required): 1. I am a employer with 4. I am a general contractor and I employees (full and/or part-time).* have hired the sub-contractors 6. New construction 2. I am a sole proprietor or partnerlisted on the attached sheet. 7. Remodeling ship and have no employees These sub-contractors have 8. Demolition working for me in any capacity. employees and have workers' 9. Building addition No workers' comp. insurance comp. insurance.‡ 5. We are a corporation and its 10. Electrical repairs or additions 3. Y I am a homeowner doing all work officers have exercised their 11, Plumbing repairs or additions myself. [No workers' comp. right of exemption per MGL 12. Roof repairs insurance required. 7 f c. 152, §1(4), and we have no employees. [No workers' 13. Other comp. insurance required.] *Any applicant that checks box #1 must also fill out the section below showing their workers' compensation policy information. Homeowners who submit this affidavit indicating they are doing all work and then hire outside contractors must submit a new affidavit indicating such. Contractors that check this box must attached an additional sheet showing the name of the sub-contractors and state whether or not those entities have ...mployees. If the sub-contractors have employees, they must provide their workers' comp. policy number. I am an employer that is providing workers' compensation insurance for my employees. Below is the policy and job site Insurance Company Name: Policy # or Self-ins, Lic. #:_______ Expiration Date:_____ Job Site Address: _____City/State/Zip: Attach a copy of the workers' compensation policy declaration page (showing the policy number and expiration date). Failure to secure coverage as required under Section 25A of MGL c. 152 can lead to the imposition of criminal penalties of a fine up to \$1,500.00 and/or one-year imprisonment, as well as civil penalties in the form of a STOP WORK ORDER and a fine of up to \$250.00 a day against the violator. Be advised that a copy of this statement may be forwarded to the Office of Investigations of the DIA for insurance coverage verification. I do hereby certify under the pains and penalties of perjury that the information provided above is true and correct. Signature: Phone #: Official use only. Do not write in this area, to be completed by city or town official. City or Town: Permit/License.# Issuing Authority (circle one): 1. Board of Health 2. Building Department 3. City/Town Clerk 4. Electrical Inspector 5. Plumbing Inspector 6. Other Contact Person:



CITY OF NEWBURYPORT BUILDING DEPARTMENT P.O. Box 550 NEWBURYPORT, MA 01950 978-465-4405

ter Binette
Ilding Commissioner/Codes Administrator

DEBRIS DISPOSAL CERTIFICATE

AFFIDAVIT

In accordance with the provisions of MGL c. 40, S 54, a condition of Building Permit Number is that the debris resulting from this work shall be disposed of in a properly licenses solid waste disposal facility as defined by MGL c. 111, S 150A.

The debris will be disposed of in:

(Location of Facility)

Signature of Permit Applicant

Date

RESTRICTIONS ON ISSUANCE OF BUILDING PERMITS MGL c. 40, S 54, Added by c. 584, S 9 of the Acts of 1987

Every city or town shall require, as a condition of issuing a building permit or license for the demolition, renovation, rehabilitation, or other alteration of a building or structure, that the debris resulting from such demolition, renovation, rehabilitation, or alteration be disposed of in a properly licensed solid waste disposal facility, as defined by section one hundred and fifty A of chapter one hundred and eleven. Any such permit or license shall indicate the location of the facility at which the debris is to be disposed. If for any reason, the debris will not be disposed of as indicated, the permitee or licensee shall notify the issuing authority as to the location where the debris will be disposed. The issuing authority shall amend the permit or license to so indicate.

THIS REQUIREMENT DOES NOT APPLY TO NEW CONSTRUCTION

In case of municipal, commercial, industrial, or multi-unit housing construction, the contractor may not know the dumpster subcontractor at the time of the building permit application. In such cases, the attached copy of an Affidavit can be used.

(b).

AFFIDAVIT

•	▼	I acknowledge that as a condition of a Building Permit, governed by this Building Permit shall be disposed of a defined by MGL c. 111, S 150A.
		oner by
	Date Sign	nature of Perinit Applicant
	(Print or type the following information)	
٠	* Stephen J McConnell	Name of Permit Applicant
	AU	Firm Name, if any
	3 Bluebery Lone, Georgebour,	NA 01833 Address
	•	

(3)

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CITY OF NEWBURYPORT BUILDING DEPARTMENT P.O. BOX 550 NEWBURYPORT, MA 01950 978-465-4405

'eter Binette ullding Commissioner/Codes Administrator

HOMEOWNER LICENSE EXEMPTION

Please Print		
Date: 11-14-2016		•
Job Location: 26 Toppaus LANC	, Lot 46	
Number Street Address	•	Section of Land
Homeowner: Steve McConnell	978-912-17	83
Name	Home Phone #	Work Phone #
Present Mailing Address: 3 Blueberry	LANE	,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,
Grovactown	MA	01833
City/Town	State	Zip Code
DEFINITION OF HOMEOWNER: 108.3.5.1 Exception Any homeowner performing work for which a building provisions of 780 CMR 108.3.5; provided that if a homeowsuch homeowner shall act as supervisor. This exception s building constructed pursuant to 780 CMR 35 and 780 "homeowner" is defined as follows: Person(s) who owns reside, on which there is, or is intended to be, a one or accessory to such use and/or farm structures. A person whe shall not be considered a homeowner.	wher engages a person(s) thall not apply to the fiel CMR R3. For the purpor a parcel of land on which two family dwelling, at	for hire to do such work, that d erection of a manufactured oses of 780 CMR 108.3.5, a h he/she resides or intends to tached or detached structures
The undersigned "homeowner" assumes responsibility fo applicable codes, by-laws, rules, and regulations.	r compliance with the S	tate Building Code and other
The undersigned "homeowner" certifies that he/she unders Department will require minimum inspection procedures a procedures and requirements.		
HOMEOWNER'S SIGNATURE:		
APPROVAL OF BUILDING COMMISSIONER:		, , , , , , , , , , , , , , , , , , ,

Note: Three family dwellings 35,000 cubic feet, or larger, will be required to comply with State Building Code Section 116.1 Construction Control.

9)

Information and Instructions

Massachusetts General Laws chapter 152 requires all employers to provide workers' compensation for their employees. Pursuant to this statute, an *employee* is defined as "...every person in the service of another under any contract of hire, "xpress or implied, oral or written."

An *employer* is defined as "an individual, partnership, association, corporation or other legal entity, or any two or more of the foregoing engaged in a joint enterprise, and including the legal representatives of a deceased employer, or the receiver or trustee of an individual, partnership, association or other legal entity, employing employees. However the owner of a dwelling house having not more than three apartments and who resides therein, or the occupant of the dwelling house of another who employs persons to do maintenance, construction or repair work on such dwelling house or on the grounds or building appurtenant thereto shall not because of such employment be deemed to be an employer."

MGL chapter 152, §25C(6) also states that "every state or local licensing agency shall withhold the issuance or renewal of a license or permit to operate a business or to construct buildings in the commonwealth for any applicant who has not produced acceptable evidence of compliance with the insurance coverage required." Additionally, MGL chapter 152, §25C(7) states "Neither the commonwealth nor any of its political subdivisions shall enter into any contract for the performance of public work until acceptable evidence of compliance with the insurance requirements of this chapter have been presented to the contracting authority."

Applicants

Please fill out the workers' compensation affidavit completely, by checking the boxes that apply to your situation and, if necessary, supply sub-contractor(s) name(s), address(es) and phone number(s) along with their certificate(s) of insurance. Limited Liability Companies (LLC) or Limited Liability Partnerships (LLP) with no employees other than the members or partners, are not required to carry workers' compensation insurance. If an LLC or LLP does have employees, a policy is required. Be advised that this affidavit may be submitted to the Department of Industrial Accidents for confirmation of insurance coverage. Also be sure to sign and date the affidavit. The affidavit should be returned to the city or town that the application for the permit or license is being requested, not the Department of Industrial Accidents. Should you have any questions regarding the law or if you are required to obtain a workers' compensation policy, please call the Department at the number listed below. Self-insured companies should enter their self-insurance license number on the appropriate line.

City or Town Officials

Please be sure that the affidavit is complete and printed legibly. The Department has provided a space at the bottom of the affidavit for you to fill out in the event the Office of Investigations has to contact you regarding the applicant. Please be sure to fill in the permit/license number which will be used as a reference number. In addition, an applicant that must submit multiple permit/license applications in any given year, need only submit one affidavit indicating current policy information (if necessary) and under "Job Site Address" the applicant should write "all locations in ______(city or town)." A copy of the affidavit that has been officially stamped or marked by the city or town may be provided to the applicant as proof that a valid affidavit is on file for future permits or licenses. A new affidavit must be filled out each year. Where a home owner or citizen is obtaining a license or permit not related to any business or commercial venture (i.e. a dog license or permit to burn leaves etc.) said person is NOT required to complete this affidavit.

The Office of Investigations would like to thank you in advance for your cooperation and should you have any questions, please do not hesitate to give us a call.

The Department's address, telephone and fax number:

The Commonwealth of Massachusetts
Department of Industrial Accidents Office of Investigations
1 Congress Street, Suite 100
Boston, MA 02114-2017
Tel. # 617-727-4900 ext. 7406 or 1-877-MASSAFE Fax # 617-727-7749
Revised 7-2013
www.mass.gov/dia



IV. WRECKING	- REMOVAL	BY DEMOLITION	– Before any	y building or	portion there	of can be
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2. Contractor	*					
License #:	A					
3. Architect or Er	ngineer: AL	Dibitso _	25 Cads	ust Ame	Spory 37	8-318-155
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EXHIBIT

BLATMAN, BOBROWSKI, MEAD & TALERMAN, LLC

30 GREEN STREET NEWBURYPORT, MA 01950 PHONE 978.463,7740 FAX 978.463,7747 Concord Office DAMONMUL SQUARE, SUTE 4A-1 (ONCORD, MA 01742 PHONE 978,371,2226 PAX 978,371,2298

> Millis Office T30 Main Street, Suite 26 Millis, Ma 02054 Phone 508,376,8400 Fan 508,376,8440

Lisa L. Mead Lisa@bbmatlaw.com

December 1, 2016

HAND DELIVERED

Peter Binette
Building Commissioner
City of Newburyport
Pleasant Street
Newburyport MA 01950

RE: Zoning Enforcement Request / 26 Toppans Lane, Donahue Court Definitive Subdivision

Dear Peter;

Reference is made to the above captioned matter. In that connection, this firm represent Virginia and Thomas Eramo (the "Eramo's") the owners of the abutting property at 28R Toppans Lane. Immediately adjacent to the Eramo's property is the approved Definitive Subdivision entitled Donahue Court and in particular Lot 4B (the "Subject Property"). The Subject Property is being developed in violation of the Definitive Subdivision Plan Decision and Plan dated May 20, 2015 said plans being dated March 28, 2015 and recorded in Plan Book 453 Plan 55 in the South Essex Registry of Deeds. (the "Definitive Plan"). On behalf of the Eramo's, we are hereby requesting, pursuant to G.L. c. 40A, § 7, that appropriate enforcement action(s) be undertaken by you.

Specifically, the current owner of the Subject Property, Steven J. McConnell, Trustee of the Stephen J. McConnell Revocable Trust, through its agents and contractors has clear cut the rear of the Subject Property and cut significant grades contrary to sheet C-2 of the Definitive Plan. Further, and contrary to note 1 on Sheet 2 of the Definitive Plan, I am informed that numerous truckloads of soils have been removed from the site. The Subject Property owner's actions have created hazardous conditions for the Eramo's in using their own property as the cut line appears to be immediately along the shared property line of the Subject Property and the Eramo's property. Further, given the significant change in grade from the rear of the Subject Property to the front of the Subject Property, the illegal cuts will likely result in significant erosion along the Eramo's property line and likely including the Eramo property. (Please see attached photographs taken from the Eramo's property)

The Definitive Decision was based upon a Stormwater Management Report which reflects the Definitive Plans. The Definitive Plans do not show complete cutting and regrading of the Subject Property which is what has occurred. To the contrary the plans show a small amount of grading around the proposed structure. Further the Stormwater Management report presumably sized the approved detention basins based upon the amount of vegetation removed and cut and fill proposed which is significantly different than what has occurred. As you can see from paragraph 11 of the General

Conditions of the Subdivision Approval (Attached hereto) "there shall be no construction other than what is shown on approved plans...unless the Planning Board shall have reviewed and approved such change." To our knowledge, no such approval has been granted. Further, paragraph 12 requires that the Developer shall take reasonable care not to disturb surrounding properties. No effort has been made to shore up the exposed edge of the cut which includes an approximate 2-4-foot vertical drop from the Eramo's property.

The Eramo's hereby request that you issue, if you have not already, a cease and desist order and an enforcement order requiring the Owner and its agents to comply with the Definitive Plan Decision and to take all necessary actions to shore up, support and make safe to life and property any exposed excavation during such time as a public hearing can be held on this matter.

Thank you in advance for your assistance. Please inform us of your enforcement action(s), or your determination that enforcement is not required and the reason(s) therefor, within the fourteen (14) days stipulated by G.L. c. 40A, § 7.

Sincerely,

Lisa L. Mead

cc: Client

Steven J. McConnell, Trustee 📝 Mark Depeiro

EXHIBIT



CITY OF NEWBURYPORT

Building Department

PETER BINETTE/BUILDING INSPECTOR

Newburyport City Hall 60 Pleasant Street P.O. Box 550 Newburyport Ma. 01950 Tel: 978-465-4405 – Fax: 978465-4452

12/8/2016

Steve McConneil
3 Blueberry Lane
Georgetown, MA 01833

RB: Site plan modification/Stop work order, 26 Toppans Lane, Lot 4b

Dear Mr. McConneil:

It has come to my attention that the site work underway at the above mentioned site may not comply with the recorded site plan approved by the Newburyport Planning Board, dated 5/20/2015. It is imperative that you contact the Newburyport Planning Office to request a modification to this site plan.

In the meantime, please consider this a stop work order at the site until The Building Department receives a release from the Pianning Office/Board as it pertains to the site conditions and elevation /drainage plan. Thank you for your anticipated cooperation in this matter. Please feel free to call me at 978 465-4405 if you have any questions.

Sincerely;

Peter Binette

Newburyport Building Commissioner

CC.

Attorney Lisa Mead Planning Director Andy Port

EXHIBIT 7



CITY OF NEWBURYPORT

DEPARTMENT OF PUBLIC SERVICES

16A PERRY WAY NEWBURYPORT, MA 01950

Anthony J. Furnari, Director Wayne S. Amaral, Deputy Director/Director of Operations

PHONE: 978-465-4463/4464 FAX: 978-465-1623

VIA CERTIFIED MAIL

December 20, 2016

Steve McConnell 3 Blueberry Lane Georgetown, MA 01833

Subject:

26 Toppans Lane - Lot 42B

STORMWATER PERMITTING VIOLATION NOTICE

Dear Mr. McConnell:

Please be advised that you are in violation with our local Stormwater Management Ordinance which requires that a Stormwater Permit be pulled for all work that disturbs 10,000 square feet or more of earth. The main purpose of this Permit is to ensure that contractors perform earthwork activities in a way that prevents erosion and degradation of our waterways. You must cease all sitework on the property until a permit has been issued and the conditions complied with.

The Stormwater Ordinance and Rules and Regulations are on our website: http://www.cityofnewburyport.com/businesses. I highly recommend that you meet with me prior to submitting the Application so we can discuss what Plans and support documents are required. This can save you valuable time. The Engineering Department issues the permit and our office is located at the address above. Once the permit is issued, fees paid, and conditions have been met, you can continue with the site work.

If you have any questions, I can be reached at 978-465-4464, x1710.

Sincerely,

NEWBERYPORT/DEPARTMENT OF PUBLIC SERVICES

Jon-Eric White, P.E.

City Engineer

CC:

Tony Furnari, DPS Director

Peter Binette, Building Commissioner

Kate Newhall-Smith, Planner

EXHIBIT

8

STORMWATER MANAGEMENT REPORT

FOR

A PROPOSED RESIDENTIAL SUBDIVISION Donahue Lane NEWBURYPORT, MASSACHUSETTS

Prepared for: 13 North Adams, LLC 9 Pasture Lane Bedford, NH 03110

Prepared by:

Design Consultants, Inc. 120 Middlesex Avenue, Suite 20 Somerville, Massachusetts 02145-1104

> Project 2014-128 March 18, 2015 Rev. April 28, 2015



TABLE OF CONTENTS

1.0	PROJECT DESCRIPTION	., 1
2.0	EXISTING CONDITIONS	1
3.0	PROPOSED CONDITIONS	. 1
4.0	ANALYSIS	2
5.0	CONCLUSION	2

FIGURES

USGS Site Location Map

APPENDICES

Appendix A: Stormwater Calculations
Appendix B: Drawings
Appendix C: Soils Information
Appendix D: Operation & Maintenance

1.0 PROJECT DESCRIPTION

The proposed project consists of two new single-family homes, cul-de-sac (hammerhead) roadway, storm water management measures, and associated utilities.

The proposed homes will be served by new underground electric, water, gas and sewer originating from Toppans Lane.

The project will disturb approximately 0.9 acres of land and construction is expected to begin in the fall of 2015 and take approximately 1 year to build.

2.0 EXISTING CONDITIONS:

The existing site condition is primarily a wooded/grass combination in fair condition, with a home and inground swimming pool. A single family home with impervious and gravel areas occupies the southeastern corner. An in-ground swimming pool and sheds are also on the property to the west of the home. Some wooded areas also follow along the northern and southern site boundaries. Soils on the land appear to be mostly class C, with an area of class A soils in the northern corner of the study area. Curve numbers and infiltrations rates for C soils were primarily used in this analysis.

Total parcel area is 64,500 ft², with approximately 26,000 ft² of that area being at least partially wooded. The site is bounded to the west by the Avita nursing facility, and to the east by Toppans Lane. Colonial Heights condominiums are located to the south of the site, and Anna Jaques Hospital lies to the north/northeast.

The entire site drains in a southeasterly direction, with slopes increasing from approximately 2% to 10% at the southeast end of the site. There is approx. thirty (30) feet of topographic relief across the site.

Field test pits conducted in the general area during the permitting process for the Avita nursing facility just north of the boundary of Lot 4. These locations are shown on the existing conditions plan, along with groundwater elevations. <u>Further soil testing will need to be performed on the project property</u>. The previous tests found that the underlying soils consisted of silt loam with indication of seasonal high groundwater within 2.3 feet of surface grade.

3.0 PROPOSED CONDITIONS:

The project consists of the construction of two (2) single family homes with a hammerhead cul-de-sac roadway and associated utilities and landscaping. Bituminous concrete will be used for the roadway and driveways. The existing home, swimming pool, and sheds on the property will be removed, and storm water management measures will be incorporated into the site to mitigate the runoff from the 2, 10, and 100-year rainfall events. Ground cover will be lawn, with some street trees proposed.

Stormwater management will be handled by the use of 2 surface detention basins and a roadside infiltration trench with a perforated HDPE pipe, eventually connecting into the municipal stormwater system in Toppans Lane. Runoff from a large portion of the property will be routed into these basins

mostly overland, with the proposed roadway being routed into the infiltration trench which is connected to the detention basins. The proposed roadway will be superelevated to drain into the trench, avoiding the need for catch basin structures in the roadway. The detention basins will have outlet pipes to mitigate the smaller storm events and allow the basins to slowly drain after rainfall ends. Exfiltration will occur in the shallow trench only, but due to the poor soils encountered on the site, an exfiltration rate of only 0.27 in/hr was factored into the calculations. Due to the relatively high groundwater elevations, surface ponds were used in place of subsurface measures. This should make maintenance simpler for the future home owners.

The drainage system has been designed so there will be no increase in runoff peak rates from the subject site once the project is complete. Runoff towards the Design Point will be below the current rates.

4.0 ANALYSIS:

The proposed drainage system was analyzed for the 2, 10, & 100-year storm events (see included calculations) to ensure that with even the most extreme storm, the proposed project would not have a negative impact on the surrounding area. The project area was analyzed in both the pre-development and the post-development conditions at the southern property limits (Design Point #1). The tables below summarize the pre-and post-development runoff rates and volumes at the Design Point:

Design Point #1

2-Year Storm / 3.10" rainfall event

	Rate of Runoff (cfs)	Volume of Runoff (AF)
Pre-development	2.28	0.208
Post-development	2.27	0.208
Percent Change	0.4%	0%

10-Year Storm / 4.7" rainfall event

	Rate of Runoff (cfs)	Volume of Runoff (AF)	
Pre-development	5.31	0,450	
Post-development	5.23	0,442	
Percent Change	1.5%	3.5%	

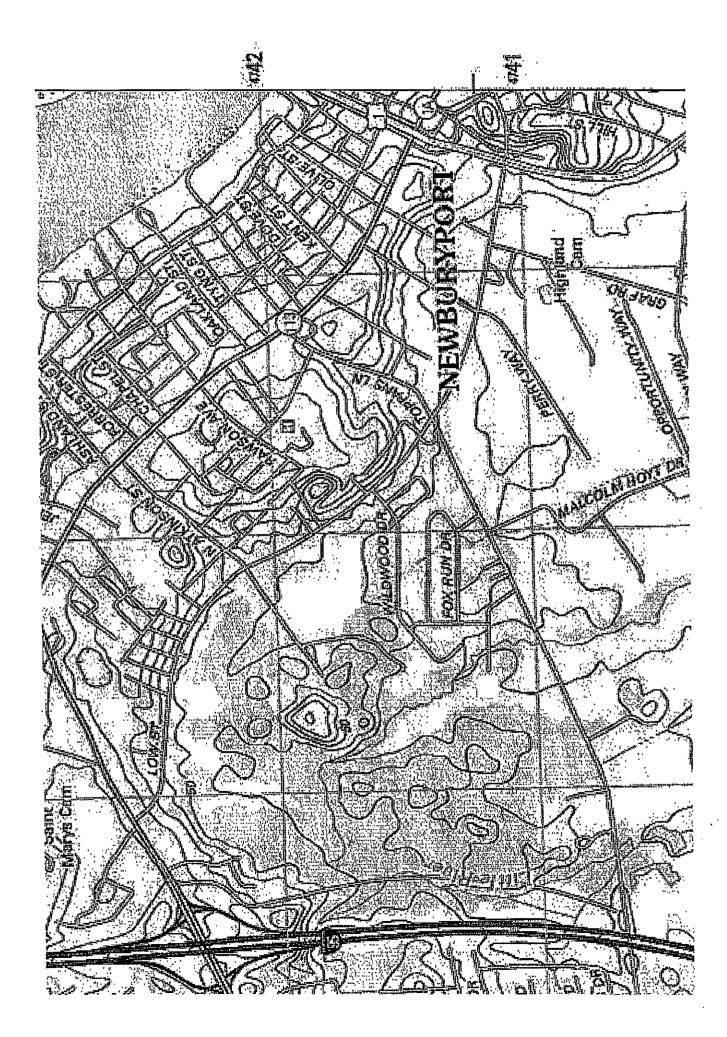
100-Year Storm / 8.3" rainfall eyent

	Rate of Runoff (cfs)	Volume of Runoff (AF)
Pre-development	13.21	1.101
Post-development	13.03	1.078
Percent Change	4.1%	2.9%

5.0 CONCLUSION:

The proposed project will disturb approximately 1.0 acres of land and result in two single-family homes being constructed. This drainage analysis examined the extreme 100 year flood storm to ensure that there would be no flooding caused downstream by the project. Existing soil conditions were examined by field inspection and previously-performed test pits in the area.

The design and analysis of the project and stormwater management plan has been consistent with MassDEP stormwater treatment and groundwater recharge techniques and guidelines. By reducing runoff through the use of surface detention basins, drainage trench, and improved ground cover, this drainage design will help to ensure that the project will not be detrimental to the environment and the surrounding properties.



Appendix A - Calculations



Existing Flow to RP



Design Point









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Page 2

Summary for Subcatchment Ext: Existing Flow to DP

Runoff =

2.28 cfs @ 12.16 hrs, Volume=

0.208 af, Depth> 0.96"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 2.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 2-year Rainfall=3.10"

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	0.5	130	0.0830	4.64		Shallow Concentrated Flow,	
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	0.5	40	0.0830	1,44		Woodland Kv= 5.0 fps	
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Summary for Reach 1R: Design Point

Inflow Area = 2.594 ac, 4.80% Impervious, Inflow Depth > 0.96" for 2-year event Inflow = 2.28 cfs @ 12.16 hrs. Volumes 0.208 af

Inflow = 2.28 cfs @ 12.16 hrs, Volume= 0.208 af Outflow = 2.28 cfs @ 12.16 hrs, Volume= 0.208 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.01 hrs

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Page 3

Summary for Subcatchment Ex1: Existing Flow to DP

Runoff 5.31 cfs @ 12.15 hrs, Volume= 0.450 af, Depth> 2.08"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 2.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10-year Rainfall=4.70"

	A	rea (sf)	CN E	escription							
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						Grass: Short n= 0.150 P2= 3.10"					
	0.5	190	0.0830	4.64		Shallow Concentrated Flow,					
			*			Unpaved Kv= 16.1 fps					
	0.7	60	0.0830	1.44		Shallow Concentrated Flow,					
						Woodland Kv= 5.0 fps					
	0,5	40	0.0830	1.44		Shallow Concentrated Flow,					
	n 4	000	0.0000	4 4 5		Woodland Kv= 5.0 fps					
	3.4	230	0.0500	1.12		Shallow Concentrated Flow,					
_	46.5		Phy. 4 . 1			Woodland Kv= 5.0 fps					
	10.5	555	Total								

Summary for Reach 1R: Design Point

Inflow Area = 2.594 ac, 4.80% Impervious, Inflow Depth > 2.08" for 10-year event

5.31 cfs @ 12.15 hrs, Volume= 5.31 cfs @ 12.15 hrs, Volume= inflow 0.450 af

Outflow 0.450 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.01 hrs

Printed 3/18/2015

Prepared by Design Consultants, Inc. HydroCAD® 9.10 s/n 00884 @ 2010 HydroCAD Software Solutions LLC

Page 4

Summary for Subcatchment Ex1: Existing Flow to DP

Runoff 13.21 cfs @ 12.15 hrs, Volume= 1.101 af, Depth> 5.10"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 2.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 100-Year Rainfall=8.30"

	Aı	rea (sf)	CN	Description							
*	,	5,314	98 E	Bidgs & par							
*		105		rick walk	•						
*		2,633			avel roads, HSG C/D						
*		51,836			oods, Fair, HSG C/D						
		5,480		Voods, Fal	r, HSG A						
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_		<u> 95,420</u>	79 5	i0-75% Gra	es cover, l	Fair, HSG C					
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	1	07,556			vious Area						
	5,419 98 4.80% Impervious Area					a .					
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	To	Length	Slope		Capacity	Description					
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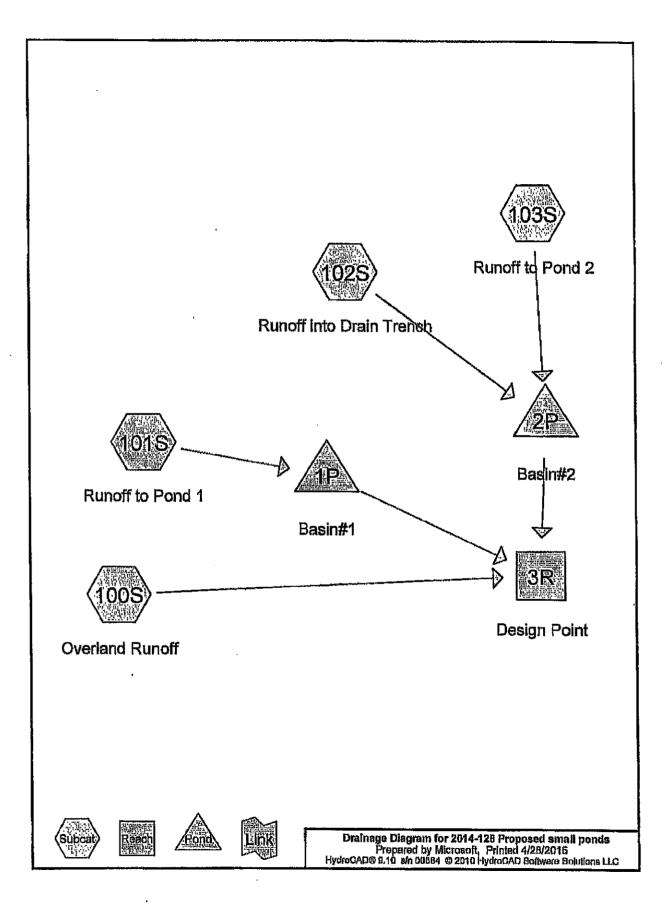
Summary for Reach 1R: Design Point

Inflow Area = 2.594 ac, 4.80% Impervious, Inflow Depth > 5.10" for 100-Year event

Inflow 1.101 at

13.21 ofs @ 12.15 hrs, Volume≔ 13.21 ofs @ 12.15 hrs, Volume≔ Outflow 1.101 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.01 hrs



2014-128 Proposed small ponds
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Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
0,123	36	Woods, Fair, HSG A (100S)
0.281	49	50-75% Grass cover, Fair, HSG A (100S, 102S)
0,642	73	Woods, Fair, HSG C (1008, 1018, 1028)
0.974	74	>75% Grass cover, Good, HSG C (100S, 101S, 102S, 103S)
0,276	79	50-75% Grass cover, Fair, HSG C (102S)
0,210	98	Impervious Areas (1005, 1025)
0.087	98	Impervious areas (101S, 103S)

2014-128 Proposed small ponds

Type III 24-hr 2-yr Rainfall=3.10" Printed 4/28/2015

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Summary for Subcatchment 100S: Overland Runoff

Runoff

=

1.25 cfs @ 12.10 hrs, Volume=

0.104 af, Depth> 0.86*

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 2.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.10"

	Δ	rea (sf)	CN	Description									
		4,982	49	50-75% Gra	ss cover, F	Fair, HSG A							
		5,379	36		Woods, Fair, HSG A								
		17,398	73		Woods, Fair, HSG C								
		30,267	74	>75% Grass	>75% Grass cover, Good, HSG C								
*		5,302	98	Impervious	Areas								
		63,328	71	Weighted A	verage								
		58,026	68	91.63% Pervious Area									
	5,302 98 8.37% Impervious Area												
	To (min)	Length (feet)	Slop (ft/f		Capacity (cfs)	Description							
	6.0			<u> </u>		Direct Entry, 6 MIN, MINIMUM							

Summary for Subcatchment 101S: Runoff to Pond 1

Runoff

¤ 0.40 cf

0.40 cfs @ 12.09 hrs, Volume=

0.031 af, Depth> 1.41"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 2.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.10"

	Ar	ea (sf)	CN	Description								
		5,166	od, HSG C									
		3,733	73	Woods, Fai	Woods, Fair, HSG C							
*		2,687	98	Impervious	areas	the letter of th						
,		11,586	79	Weighted Average								
		8,899	74	76.81% Pervious Area								
		2,687	98	23,19% lmp								
(Tc min)	Length (feet)	Slop (ft/f		Capacity (cfs)	Description						
	6.0					Direct Entry, 6 min. minimum						

Summary for Subcatchment 102S: Runoff into Drain Trench

Runoff :

0.74 cfs @ 12.10 hrs, Volume=

0.060 af, Depth> 0.98"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 2.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 2-yr Rainfall=3.10"

2014-128 Proposed small ponds

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Type III 24-hr 2-yr Rainfall=3.10" Printed 4/28/2015

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	Area (sf)	CN	Description									
	7,276	49	50-76% Gre	0-76% Grass cover, Fair, HSO A								
	12,022	79		50-75% Grass cover, Fair, HSG C								
	2,220	74	>75% Grass									
	6,817	73	Woods, Fal	HSG C	·							
*	3,85 0	98	<u>Impervious</u>	Áreas								
,	32,185	73	Weighted Average									
	28,335	69	88.04% Pervious Area									
	3,850	98	11.96% lmp	ervious An								
-	Γc Length	Slope	e Velocity	Capacity	Description							
<u>(mi</u>	n) (feet)	(ft/ft) (ft/sec)	(cfs)								
6	.0		·		Direct Entry,							

Summary for Subcatchment 103S: Runoff to Pond 2

Runoff

0.19 cfs @ 12.09 hrs, Volume=

0.015 af, Depth> 1.33"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 2.00-24.00 hrs, di= 0.01 hrs Type III 24-hr 2-yr Rainfall≔3.10"

	A	rea (sf)	CN	Description								
		4,778	74	>75% Gras	75% Grass cover, Good, HSG C							
*		1,100	98	impervious	mpervious areas							
-		5,876	78	Weighted A	Veighted Average							
		4,776	74	81.28% Pervious Area								
		1,100	98	18.72% Impervious Area								
	Tc (min)	Length (feet)	Slope (ft/fl		Capacity (cfs)	Description						
	6.0					Direct Entry, 6 min. minimum						

Summary for Reach 3R: Design Point

2.594 ac, 11.45% impervious, inflow Depth > 0.96" for 2-yr event Inflow Area ⊨

2.27 cfs @ 12.11 hrs, Volume= 2.27 cfs @ 12.11 hrs, Volume= Inflow 0.208 af

Outflow 0.208 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.01 hrs

Summary for Pond 1P: Basin#1

Inflow Area	a =	0.266 ac, 2	23.19% Impervious	, Inflow Depth >	1.41"	for 2-vre	vent
Inflow	秤		12.09 hrs. Volum			•	
Chifficase	=	11 32 cfs @	12 15 hrs. Volum	0.020	\ mF AH	on~ 200/ I	1 2 (

0.32 cfs @ 12.15 hrs, Volume= 0.32 cfs @ 12.15 hrs, Volume= 0.030 af, Atten= 22%, Lag= 3.9 min

Primary 0.030 af

Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 56.63' @ 12.15 hrs Surf.Area= 367 sf Storage= 187 cf

Plug-Flow detention time= 63.7 min calculated for 0.030 af (95% of Inflow)

Type III 24-hr 2-yr Rainfall=3.10" Printed 4/28/2015

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Center-of-Mass det. time= 27.5 mln (840.6 - 813.1)

Volume	lm	ert Avail.Sto	rage Storage	Description				
#1	56.	.00' 9			c)Listed below (Rec	aic)		
Elevation (feet)		Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet,Area			
56,1 57,1 58,1	00	233 460 740	0 340 594	0 340 935	233 469 762			
Device	Routing	Invert	Outlet Devices	1				
#1	Primary	56.20'	6.0" Round Culvert L= 15.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 56.20' / 56.00' S= 0.0133 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth Interior					

Primary OutFlow Max=0.32 cfs @ 12.15 hrs HW=56.63' TW=0.00' (Dynamic Tailwater) 1=Culvert (Inlet Controls 0.32 cfs @ 1.76 fps)

Summary for Pond 2P: Basin#2

Inflow Area = 0.874 ac, 13.01% Impervious, Inflow Depth > 1.03" for 2-yr event 0.93 cfs @ 12.09 hrs, Volume= 0.78 cfs @ 12.15 hrs, Volume= 0.78 cfs @ 12.15 hrs, Volume= Inflow 0.075 af Outflow 0.074 af, Atten= 17%, Lag= 3.3 min Primary 0.074 af

Routing by Dyn-Stor-Ind method, Time Span≃ 2.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 50.75' @ 12.15 hrs Surf.Area= 381 sf Storage= 219 cf

Plug-Flow detention time= 21.5 min calculated for 0.074 af (98% of Inflow) Center-of-Mass det. time= 11.7 min (845.8 - 834.1)

Votume	Im	<u>vert</u> Avail,8	Storage Storag	ge Description					
#1	50.	00' 1		om Stage Data (Con	ic)Listed below	(Recald)			
Elevation (feet)		Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (oubic-feet)	Wet,Area (sq-ft)				
50.0 51.0 52.2	00	214 448 740	0 324 706	0 324 1,029	214 456 766				
Device	Routing	Inve	rt Outlet Devi	oes:					
#1 #2	Primary Primary		0" 11.0" Horiz	9.5" W x 4.0" H Vert. Orifice/Grate C= 0.600 11.0" Horlz. Orifice/Grate C= 0.600 Limited to weir flow at low heads					

Primary OutFlow Max=0.78 cfs @ 12.15 hrs HW=50.75' TW=0.00' (Dynamic Tailwater)

-1=Orifice/Grate (Orifice Controls 0.78 cfs @ 2.94 fps)

-2=Orifice/Grate (Controls 0.00 cfs)

Type III 24-hr 10-yr Rainfall=4.70" Printed 4/28/2015

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Summary for Subcatchment 100S: Overland Runoff

Runoff

3.07 cfs @ 12.09 hrs, Volume=

0.230 af. Depth> 1.90"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 2.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfail=4,70"

	Α	rea (sf)	CN	Description						
	, -	4,982	49	50-75% Gra	0-75% Grass cover, Fair, HSG A					
		5,379	36	Woods, Fair, HSG A						
		17,398	73	Woods, Fair, HSG C						
		30,267	74	>75% Grass	>75% Grass cover, Good, HSG C					
*		5,302	98	impervious	Areas					
		63,328	71	Weighted A	verage					
		58,026	68	91.63% Per	vlous Area					
		5,302	98	8.37% Impe	rvious Area	a e				
	Tc	Length	Slop	e Velocity	Capacity	Description				
-	(min)	(feet)	(ft/f	t) (fl/sec)	(cfs)	•				
,,,,,	6,0					Direct Entry, 6 MIN. MINIMUM	 -			

Summary for Subcatchment 101S: Runoff to Pond 1

Runoff

0.79 cfs @ 12.09 hrs, Volume=

0.059 af, Depth> 2.66"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/imperv., Time Span= 2.00-24.00 hrs, di= 0.01 hrs Type III 24-hr 10-yr Rainfall=4,70"

	A	rea (sf)	ÇN	Description						
		5,166	74	>75% Gras	s cover, Go	od, HSG C				
		3,733	73	Woods, Fai	r, HSG C	.`				
*		2,687	98	impervious	mpervious areas					
		11,686	79	Weighted A	eighted Average					
		8,899	74	76.81% Pervious Area						
		2,687	88	23.19% lmp	ervious Are	98				
<u>(ı</u>	To min)	Length (feet)	Slope (ft/ft)		Capacity (cfs)	Description				
	6.0					Direct Entry, 6 min. minimum				

Summary for Subcatchment 102S: Runoff into Drain Trench

Runoff

1.69 cfs @ 12.09 hrs, Volume=

0.127 af, Depth> 2.06"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 2.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfali=4.70"

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	Area (sf)	CN	Description								
•	7,276	49	50-75% Gra	0-75% Grass cover, Fair, H8G A							
	12.022	79	50-75% Gra	50-75% Grass cover, Fair, HSG C							
	2,220	74	>75% Grass cover, Good, HSG C								
	6,817	73	Woods, Fair	Noods, Fair, HSG C							
*	3,850	98	Impervious /	mpervious Áreas							
	32,185	73	Weighted Average								
	28,335	69	88.04% Per	vious Area							
	3,850	88	11.96% lmp	ervious Are	∂a						
T (min		Slop (ft/i		Capacity (cfs)	Description						
6.					Direct Entry,						

Summary for Subcatchment 103S: Runoff to Pond 2

0.39 ofs @ 12.09 hrs, Volume¤ Runoff

0.029 af. Depth> 2.56"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 2.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10-yr Rainfall=4.70"

	Α	rea (sf)_	CN	Description						
_		4,778	74	>75% Grass	a cover, Go	od, HSG C				
*		1,100_	98	Impervious	areas		<u></u>			
-		5,876	78	Weighted A	verage					
		4,776	74	81,28% Pervious Area						
		1,100	98	18.72% lmp	ervious An	38				
	Tc (min)	Length (feet)	Slop (ft/f		Capacity (cfs)	Description				
-	6.0					Direct Entry, 6 mln. minimum				

Summary for Reach 3R: Design Point

2.594 eq. 11.45% Impervious, Inflow Depth > 2.05" for 10-yr event Inflow Area =

0.442 af Inflow

5.23 cfs @ 12.12 hrs, Volume= 5.23 cfs @ 12.12 hrs, Volume= 0.442 af, Atten= 0%, Lag= 0.0 min Outflow

Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.01 hrs

Summary for Pond 1P: Basin#1

0.266 ac, 23.19% Impervious, Inflow Depth > 2.86" for 10-yr event Inflow Area =

0.79 cfs @ 12.09 hrs, Volume= 0.53 cfs @ 12.18 hrs, Volume= 0.53 cfs @ 12.18 hrs, Volume= 0.059 af Inflow

0.057 af, Atten= 32%, Lag= 5.2 min Outflow

0.057 af Primary

Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 56,96 @ 12.18 hrs Surf.Area= 450 sf Storage= 322 cf

Plug-Flow detention time= 35.1 min calculated for 0.057 af (97% of inflow)

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Type III 24-hr 10-yr Rainfall=4.70" Printed 4/28/2015

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Center-of-Mass det. time= 19.6 min (824.5 - 804.9)

<u>Volume</u>	Inv	ert Avall.Sto	rage Storage D	Description				
#1	56.	00' 9:	35 cf Custom	Stage Data (Conl	c)Listed below (F	Recalc)		
Elevatio (fee		Surf.Area (sq-ft)	inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)			
56.0 57. 0 58.0)0	233 460 740	0 0 340 340 594 936		233 469 762			
Device	Routing	Invert	Outlet Devices					
#1	Primary	56.20'	6.0" Round Culvert L= 15.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet invert= 56.20' / 56.00' S= 0.0133 '/' Cc= 0.900 n= 0.013 Corrugated PF, smooth interior					

Primary OutFlow Max=0.53 cfs @ 12.18 hrs HW=56,96' TW=0.00' (Dynamic Tailwater) 1=Culvert (Inlet Controls 0.53 cfs @ 2.72 fps)

Summary for Pond 2P: Basin#2

Inflow Area = 0.874 ac, 13.01% Impervious, Inflow Depth > 2.14" for 10-yr event lnflow = 2.08 cfs @ 12.09 hrs, Volume= 0.156 af Outflow = 1.85 cfs @ 12.13 hrs, Volume= 0.155 af, Atten=11%, Lag= 2.5 min Primary = 1.85 cfs @ 12.13 hrs, Volume= 0.155 af

Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 51.36' @ 12.13 hrs Surf.Area= 527 sf Storage= 498 cf

Plug-Flow detention time= 13.3 min calculated for 0.154 af (99% of inflow) Center-of-Mass det. time= 8.0 min (831.4 - 823.4)

<u>Volume</u>	Inv	ert Avail.Sto	rage Storage	Description			
#1	50.	0,1 '00	29 cf Custom	Stage Data (Coni	c)Listed below	(Recalc)	
Elevatio		Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)		
50.0 51.0 52.2	00	214 448 740	0 324 706	0 324 1,029	214 456 766		
Device	Routing	Invert	Outlet Devices	<u>u</u>			
#1 #2	Printery Primery		9.5" W x 4.0" H Vert. Orifice/Grate C= 0.600 11.0" Horiz. Orifice/Grate C= 0.600 Limited to weir flow at low heads				

Primary OutFlow Max=1.85 cfs @ 12.13 hrs HW=51.36' TW=0.00' (Dynamic Tailwater)

—1=Orifice/Grate (Orifice Controls 1.26 cfs @ 4.78 fps) —2=Orifice/Grate (Weir Controls 0.58 cfs @ 1.29 fps)

Type III 24-hr 100-yr Rainfall=8.30" Printed 4/28/2015

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Summary for Subcatchment 100S: Overland Runoff

Runoff

8.00 cfs @ 12.09 hrs, Volume=

0.579 af. Depth> 4.78"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 2.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfali=8.30"

	Α	rea (sf)	CN	Description		•			
		4,982	49	50-75% Gra	ass cover. F	Fair, HSG A			
		5,379	36	Woods, Fai					
		17,398	73	Woods, Fai					
		30,267	74	>75% Grass cover, Good, HSG C					
*		5,302	98	Impervious	Areas				
		63,328	71	Weighted A	verage				
		58,026	68	91.63% Per		9			
		5,302	98	8.37% Impe					
(t	Tc min)	Length (feet)	Slop (ft/i		Capacity (cfs)	Description			
	6.0					Direct Entry, 6 MIN, MINIMIN			

Summary for Subcatchment 1018: Runoff to Pond 1

Runoff

1.74 cfs @ 12.09 hrs, Volume=

0.130 af, Depth> 5.84"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 2.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfail=8.30"

	A	rea (sf)	ÇN	Description							
		5,166	74	>75% Grass cover, Good, HSG C							
		3,733	73	,							
*		2,687	98	Impervious:	areas						
-	. ,	11,586	79	Weighted A	verage						
		8,899	74	76.81% Pervious Area							
		2,687	98	23.19% lmp	ervious An	ea					
(r	Tc min)	Length (feet)	Slop (ft/f		Capacity (ofs)	Description					
	6,0				.,	Direct Entry, 6 min, minimum					

Summary for Subcatchment 102S: Runoff into Drain Trench

Runoff

4.23 cfs @ 12.09 hrs, Volume=

0.308 af, Depth> 5.01"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 2.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfali=8.30"

Type III 24-hr 100-yr Rainfall≔8.30"

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	Α	rea (sf)	CN	Description					
		7,276	49	50-75% Gre	14444				
		12,022	79	50-75% Grass cover, Fair, HSG C					
		2,220	74	>75% Grass cover, Good, HSG C					
		6,817	73						
*		3,850	98	Impervious	Areas				
		32,185	73	Weighted Average					
		28,335	69	88.04% Per					
		3,850	98	11.96% lmp		9a			
,	To (min)	Length (feet)	Slop (ft/f		Capacity (cfs)	Description			
		(100t)	tivi	(10866)	(018)				
	6.0					Direct Entry,			

Summary for Subcatchment 103S: Runoff to Pond 2

Runoff

0,87 cfs @ 12.09 hrs. Volume=

0.064 af. Depth> 5.72"

Runoff by SCS TR-20 method, UH=SCS, Split Pervious/Imperv., Time Span= 2.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 100-yr Rainfali=8,30"

	Area (sf)	CN	Description						
	4,776	74	>75% Gras	s cover, Go	od, HSG C				
*	1,100	98	<u>impervious</u>	areas					
	5,876	78	Weighted A	eighted Average					
	4,776	74	31.28% Pervious Area						
	1,100	98	18.72% lmp	ervious Ar	98				
To (mln)		Slope (ft/ft		Capacity (cfs)	Description				
6.0			,,,,,,		Direct Entry, 6 min. minimum				

Summary for Reach 3R: Design Point

Inflow Area =

2.594 ac, 11.45% impervious, Inflow Depth > 4.99" for 100-yr event 1.078 af

13.03 cfs @ 12.10 hrs, Volume= 13.03 cfs @ 12.10 hrs, Volume= Inflow Outflow

1.078 af, Atten= 0%, Lag= 0.0 min

Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt≈ 0.01 hrs

Summary for Pond 1P: Basin#1

0.266 ac, 23.19% Impervious, Inflow Depth > 5.84" for 100-yr event Inflow Area =

Inflow 0.130 af

1.74 cfs @ 12.09 hrs, Volume= 0.89 cfs @ 12.23 hrs, Volume= 0.89 cfs @ 12.23 hrs, Volume= Outflow 0.128 af, Atten= 49%, Lag= 8.6 min

Primary 0.128 af

Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 57.87 @ 12.23 hrs Surf.Area= 699 sf Storage= 838 cf

Plug-Flow detention time= 23.3 min calculated for 0.128 af (99% of inflow)

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Type III 24-hr 100-yr Rainfail=8.30" Printed 4/28/2015

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Center-of-Mass det. time= 15.0 min (806.7 - 791.7)

Volume	Inv	ert Avall.Sto	rage Storage	Description		
#1	56.	00, 8	35 cf Custom	Stage Data (Coni	c)Listed below (Recalc)
Elevetic		Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
56.0 57.0 58.0	00	233 460 740	0 340 594	0 340 935	233 469 762	
Device	Routing	Invert	Outlet Devices	3		
#1	Primary	56.20'	Inlet / Outlet in	Culvert ', projecting, no he nvert= 56.20' / 56.0 rugated PE, smoot	0' S= 0.0133 V	900 " Cc= 0.900

Primary OutFlow Max=0.89 cfs @ 12.23 hrs HW=57.87' TW=0.00' (Dynamic Tailwater)
1=Culvert (Inlet Controls 0.89 cfs @ 4.52 fps)

Summary for Pond 2P: Basin#2

Inflow Area =	0.874 ac, 13.01% Impervious, inflow D	Depth > 5.12" for 100-vrevent
inflow ⊨	5.10 cfs @ 12.09 hrs, Volume=	0.373 af
Outflow =	4.46 cfs @ 12.13 hrs, Volume≔	0.371 af, Atten= 13%, Lag= 2.7 min
Prlmary =	4.46 cfs @ 12.13 hrs, Volume=	0.371 af

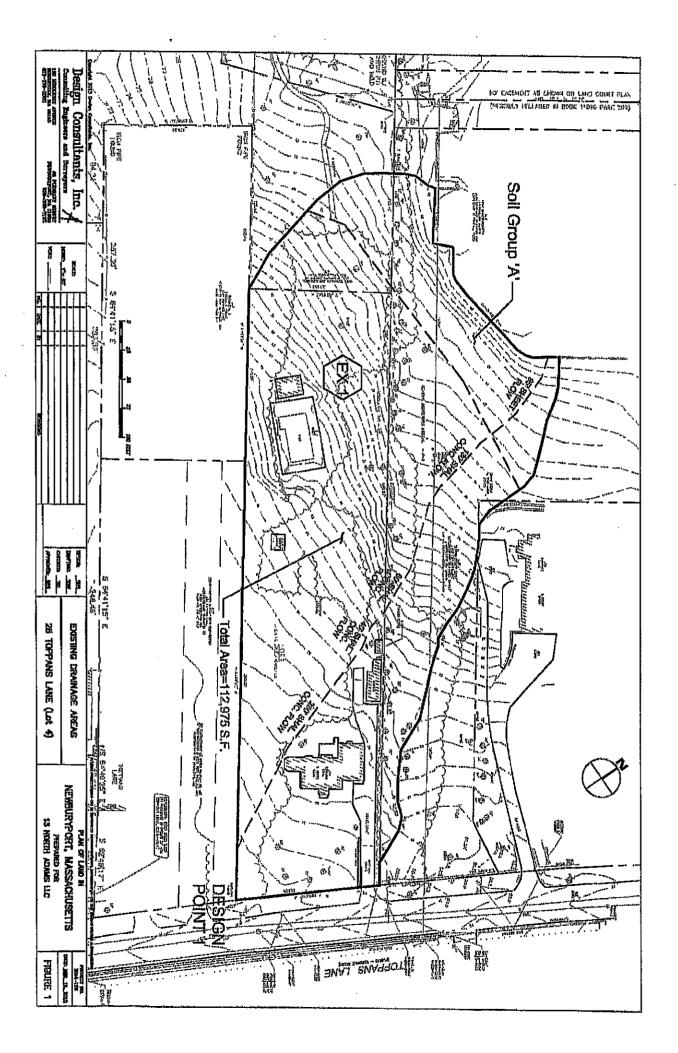
Routing by Dyn-Stor-Ind method, Time Span= 2.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 52.00' @ 12.13 hrs Surf.Area= 686 sf Storage= 886 cf

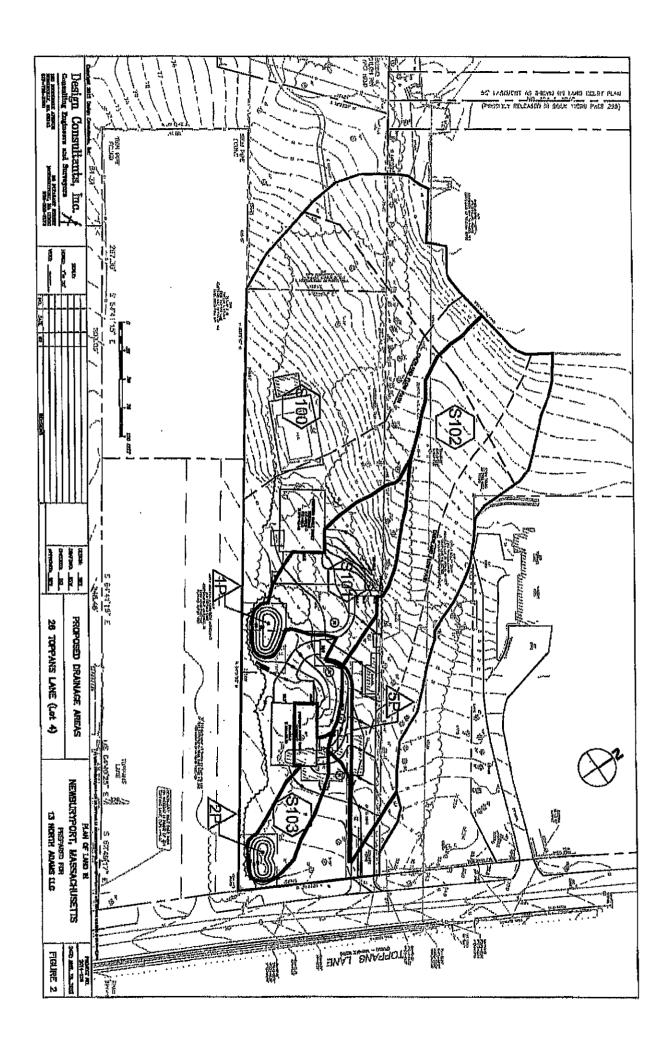
Plug-Flow detention time= 7.7 mln calculated for 0.371 af (100% of Inflow) Center-of-Mass det. time= 5.1 mln (811.9 - 806.8)

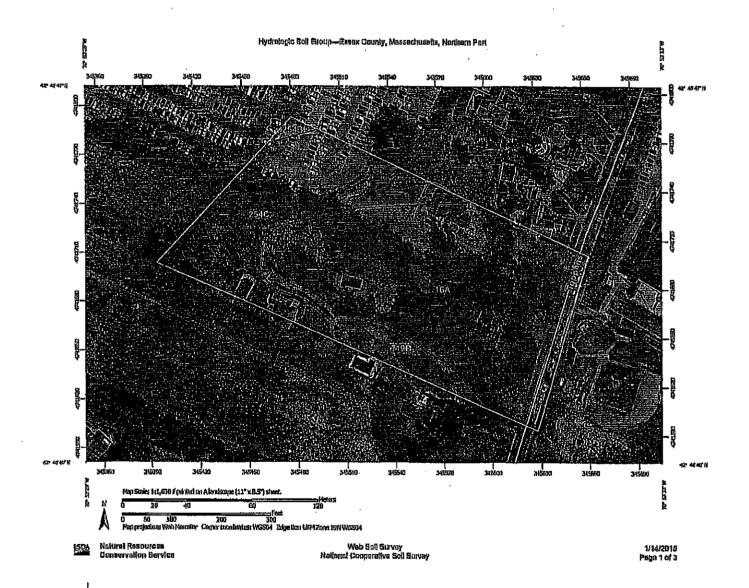
Volume	Inv	ert Avail.Sto	rage Storage	Description		
#1	50,	00' 1,0	29 of Custom	Stage Data (Coni	c)Listed below	(Recalc)
Elevation (fee		Surf.Area (sq-ft)	inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
50.0 51.0 52.2	00	214 448 740	0 324 706	0 324 1,029	214 456 766	
Device	Routing	Invert	Outlet Devices	i		
#1 #2	Primary Primary	50.20' 51.20'	11.0" Horiz. C	H Vert. Orifice/Gr Prifice/Grate C= (flow at low heads	0.600	

Primary OutFlow Max=4.46 ofs @ 12.13 hrs HW=52.00' TW=0.00' (Dynamic Tallwater)

1=Orifice/Grate (Orifice Controls 1.62 cfs @ 6.15 fps) 2=Orifice/Grate (Orifice Controls 2.84 cfs @ 4.30 fps)







MAP LEGEND MAP INFORMATION Area of interest (ADI) The sell surveys that complete your AOI ware mapped at 1:15,000. Arou of interest (AOI) OJD Warning: Soil Map may not be volid at this scale. Sojin D Ū Enlargement of maps beyond the scale of mapping cen cause utaunderstanding of the detail of mapping and accuracy of soil line placement. The maps do not show the small cross of contracting soils that could have been abover at a maps detailed scale, Golf Hating Polygons Not ented or not avellable П A Water Pastures 匨 A/D . Streams and Canals 鰮 8 Transpariation Please rely on the bur scale on such map sheet for map 靈 ĐŒ mozaujements. Floids +++ C == Source of Map: Natural Resources Conservation Service Interptiste Highwaya Web Soil Burvey Lift: http://websulleurvey.nrcs.uade.gov Coordinate System: Web Morretor (EPSG:3857) 性 0/0 US Roules Ħ 壐 Maps from the Web Soil Survey are based on the Web Mercetor projection, whilst preserves direction and shape but distorie distance and area, A projection that preserves area, such as the Albara aqual-ayas contegrajection, afould be used if more accurate Miljor Rhads :Name 35. Not raied or not available Local Roads era. Soil Railing Lines Background Α Awital Photography colculations of distance or erea are required. a ... ΑD This product to generated from the USDA-NRCS confided data as of ihn version dela(s) listed below. Ħ Boll Servey Area: Easox County, Massachusella, Northern Parl Burvey Area Dete: Version 10, Sep 19, 2014 ED 8 Soli map unils are labeled (as space allows) (or map scales 1)50,000 0/0 ar larger, D Not ruted or not available The orthopholo or other bess map on which the soil thes were complied and righted probably differs from the budground imagery displayed on these maps. As a result, some minor shifting of map wit bountaries may be evident. Boll Halling Patate F Α'n Ш Ħ B/D

Hydrologic Soil Group

-iydrologic	Soll Group—Summary by a	Ap Unit — Essex County	, Massachusetts, Northe	rn Pert (MA606)
Map unit symbol	Map unit name	Rating	Acres in AO	Percent of AO
10A	Sceniic silt loam, 0 to 3 percent slopes	C/D	4,0	81.7%
2648	Merrimae fine sandy Joam, 3 to 8 percent slopes	A	0.0	0.7%
254C	Merrimae fine sandy loam, 8 to 16 percent stopes	A	2,0	31.2%
7199	Suffield silt loam, 3 to 8 percent slopes	C	0,4	6,4%
Totals for Area of Inte	ornat	, W-1 , L	6.4	100.0%

Rating Options

Aggregation Method: Dominant Condition
Component Percent Cutoff: None Specified

Tie-break Rule: Higher



Appendix D - Operation & Maintenance Plan

STORMWATER MANAGEMENT OPERATION AND MAINTENANCE PLAN

Donahue Lane Newburyport, Massachusetts

The following Stormwater Management Operation and Maintenance (O&M) Plan has been prepared to operate and maintain the stormwater management system for Donahue Lane.

Owner/Operator:

13 North Adams, LLC

9 Pasture Lane Bedford, NH 03110

Inspection and Maintenance Schedule

Facility personnel will inspect the storm water management system on a routine basis not less than once per month for the first 6 months of operation and annually thereafter. Refer to Sheet C-2, Grading & Drainage Plan, for drainage structure locations. Inspection and maintenance shall be performed as follows:

- Roadside Gravel Infiltration Trench shall be inspected for accumulation of silt, sediment, standing water, or debris on a semi-annual basis at a minimum, and after every rainfall event of 2-inch or more. Observations shall be made via the observation well built into the system to ensure that the pipe has completely drained 24 hours after the storm event has ended. If standing water is observed more than 24 hours after any sized rain event that infiltration system is deemed failed and requires replacement. In the event of an infiltration system failure, the crushed stone and pipe will need to be removed and replaced. The non-woven filter fabric that surrounds the trench will need to be disposed of and replaced.
- 2. <u>Landscaped Areas</u> shall be inspected and maintained on a regular basis. Areas which may be subject to erosion will be stabilized and reseeded immediately. These operations will be performed as part of ongoing routine grounds maintenance operations.
- Street Sweeping of drives and roadway shall be conducted bimonthly between the months
 of April and November. Removed sediment will be disposed off site by a qualified waste
 disposal contractor in accordance with state and federal regulations.
- 4. <u>Detention Areas</u>: Vegetation shall be inspected monthly for disease or pest problems. If treatment is warranted, use the least toxic approach. Promptly replace any vegetation that is beyond treatment. During times of extended drought, inspect vegetation for signs of stress including wilting or spotted or brown leaves. Water as required. Detention areas shall be weeded at least once a year as required.

Inspect soil and repair eroded areas monthly. Re-plant void areas as needed. Remove litter and debris monthly. Remove and replace dead vegetation twice per year in spring and fall.

Replace soil media if ponding is witnessed more than 48 hours after rainfall event within the surface ponding area.

Inspection and Maintenance Procedures for Outlet Control Structure (Flat Top Precast Manhole)

Sump shall be inspected quarterly during the first year to determine sediment collection. Sump shall be cleaned annually at a minimum, or when sediment and debris are within 1-feet deep. Additionally there should be periodic inspections of the structure and surrounding areas for pollutants such as leaks from dumpsters, minor spills, and dumping and litter. If pollutants are found, action should be taken immediately to have the pollutant source removed.

All sediments removed from the outlet control structure sump shall be disposed of properly, and in accordance with all applicable local and state regulations,

Stormwater System Inspection Report General Information Location: Donahue Lane, Newburyport **Date of Inspection** Start/End Time Inspector's Name(s) Inspector's Title(s) Inspector's Contact Information Purpose of Inspection Weather Information Has it rained since the last inspection? TYOS UNO Weather at time of this inspection? Construction Phase Erosion/Sedimentation Control Measures installed and Date for Corrective Action/Responsible Operating ! Description Corrective Action Needed Properly? Stabilized Construction □Yes □No Entrance Silt Sacks in stormwater □Yes □No Inlets 3 **Erosion Control Barriers** □Yes □ND Sediment tracking in □Yes □No roadway(s) □Yes □No Yehicle wash area Concrete washout aren □Yes □No

Permanent Site-Specific Stormwater Devices

Installed and Date for Correcti	
Operating Action/Responsib Property? Corrective Action Needed Person	

	Déscription :=	Installed and Operating Properly?	Corrective Action Needed	Date for Corrective Action/Responsible
1	SZBENTANÍA WIME#WWW.	□Yes □No	Sovvectives venou / seeded	Person
2		□Yes □No	7,44	
3	W-11-12-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-1-	UYes DNo		
4		□Yes □No	Mary to the second seco	
5		□Yes □No		
6	t de la desta dela dela dela dela dela dela dela del	UYes UNo		***
7		□Yes □No		
8		□Yes □No	Name of the state	
9		□Yes □No		
10		□Yes □No		

Description	Installed and Operating Property ((Corrective Action) Needed:	Date for Corrective Action/Responsible Person
11	□Усв □Nо		
12	QYes QNo		
13	□Yes □No		
14	UYes □No		
15	□Yes □No		
16	□Yes □No		, , , , , , , , , , , , , , , , , , ,
17	□Yes □No		

		Installed i	and		Date for Corrective
	Description	Operating Property?		Corrective Action Needed	Action/Responsible
18		□Yes □	No	SOLVERY CALIFORNIA DE LICONOMIA	Ecison Same Service
19		□Yes □	No		711
20		□Yes □	No		
21		□Yes □	No		
22		□Yes □	No		
23		□Yes □	No		
24		□Yes □	No		
25		□Yes □	No		The state of the s
26		□Yes □	No		
27	W10.	□Yes □	No		
28		□Yes □	No		, , , , , , , , , , , , , , , , , , ,
29		□Yes □	No		
30		□Yes □	No		
Ove	rall Site Issues		enwen er i		
712-1-1	Description		e C	orvediyeA'dlon	Date for Corrective Action/Responsible Rerson
1	Are all slopes properly stabilized?	□Yes □N			The state of the s
2	Are natural resource areas (e.g., streams, wetlands, etc.) being subjected to erosion?	□Yes □ì	No		
3	Are discharge points free of sediment deposits?	DYes Di	No l		

Certification Statement:

"I certify under penalty of law that this document and all attachments were prepared under my direction or supervision in accordance with a system designed to assure that qualified personnel properly gathered and evaluated the information submitted. Based on my inquiry of the person or persons who manage the system, or those persons directly responsible for gathering the information, the information submitted is, to the best of my knowledge and belief, true, accurate, and complete. I am aware that there are significant penalties for submitting false information, including the possibility of fine and imprisonment for knowing violations."

Print name	
Signature:	· ·
Date:	



STORMWATER MANAGEMENT PERMIT

APPLICATION

isiti E

	Revised Q5/15/14
	m le lim
ecelved Date:	216117
Fee Paid:	227,00
Date Paid:	2/6/17
Permit #:	SMP-00
Approved By:	JEW.
pproval Date:	2/8/17
	(For DPS use only)

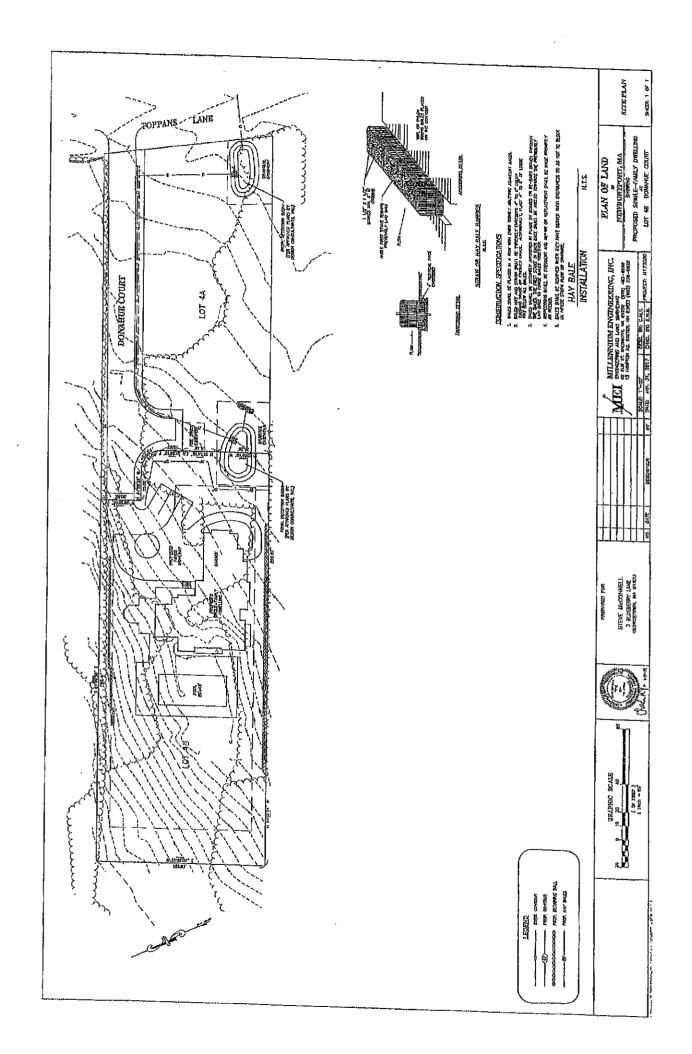
Department of Public Services Engineering Department 16A Perry Way Newburyport, MA 01950 Telephone: 978-465-4464 x1711

1.	Project / Site Informa	tion							
	Is site less than 10,00	s site less than 10,000 sq ft of land disturbance? If 'no', then no permit required. If 'yes', continue below.							
	Project / Site Name: _	Project / Site Name: Donahue Cours							
	Project Street / Location: 26 Toppins Line, Lot 46 Newbrygout MA 01								
			7	, , , , , , , , , , , , , , , , , , , ,					
	Applicant Type (Check	One) Single-Family	Commercial and Othe	r Non-Single-Family					
	Applicati	on Fee Structure	Proposed Project Land Disturbance (sq. ft.)	Application Fee (Non-Refundable)					
	Land disturbance less than 10,000 square feet	No permit required	27,000	\$227					
:	Land disturbance 10,000 square feet and greater \$200.00 base fee plus \$1.00 for every 1,000 square feet of land disturbance		2 1,000						
	Total Area of Imperviol (Paved, parking, deck			<u>Net</u> 9, 298					
2.	2. Applicant Information 3. Owner Information **E Check box if Owner is also the Applicant								
	me: Steve	McConney	•						
Αd		Ty Leve biogetimn 4	A.						
Ph	one: 978-913		Q 2.5						
E-1	mall: + lutic	3/6 gmail.com							

4. Application Waiver

The project described above is exempt from meeting the stormwater management standards as outlined in the Newburyport Stormwater Management Ordinances (Chapter 17) for the following reason:

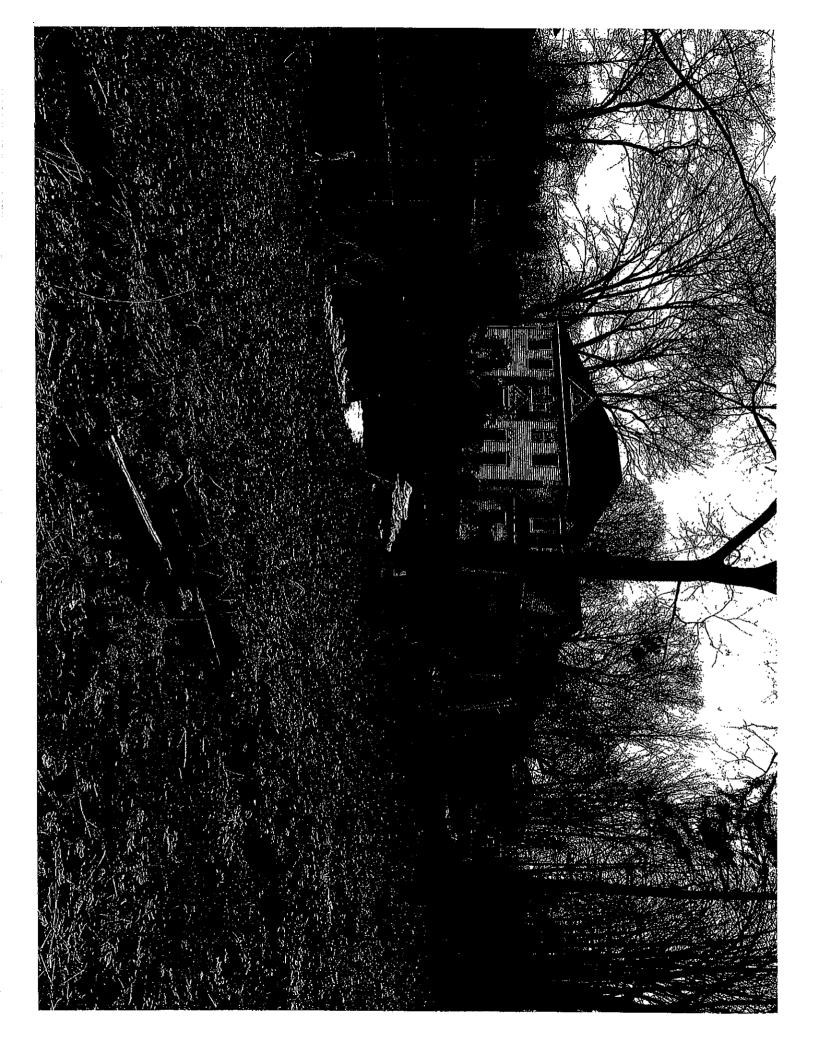
☐ Land disturbance is less than 10,000 square feet.



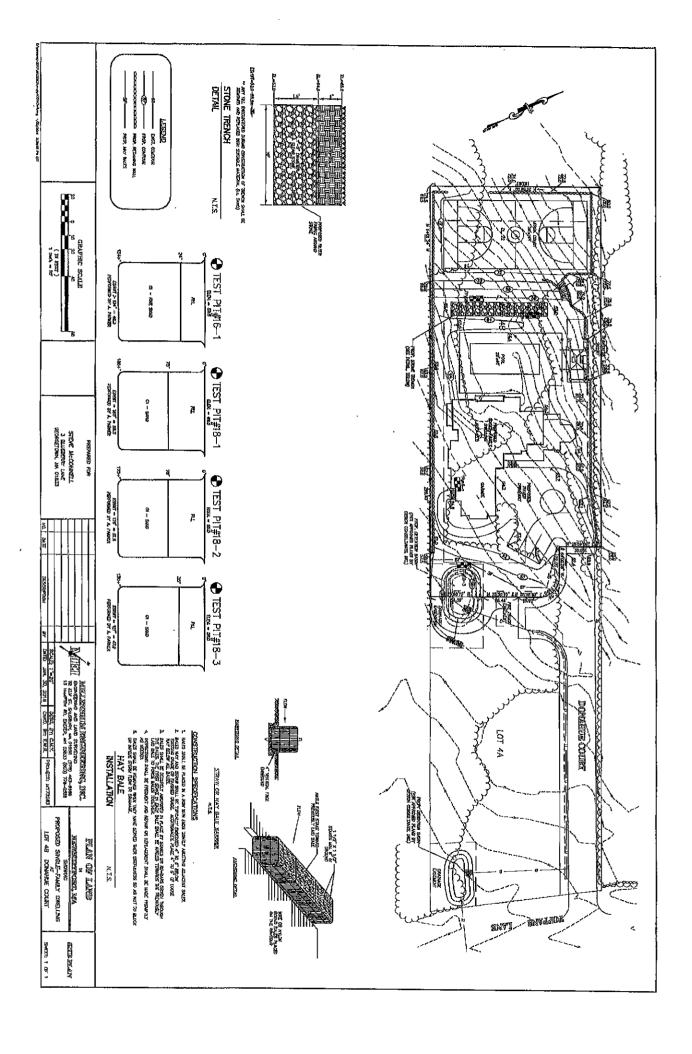
10







11



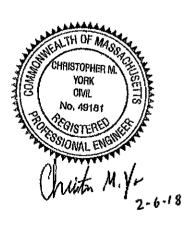
STORMWATER CALCULATIONS

FOR: STEVE McCONNELL
PROPOSED SINGLE FAMILY DWELLING
LOT 4B DONAHUE COURT
NEWBURYPORT, MA

PREPARED BY:

MILLENNIUM ENGINEERING, INC. 62 ELM STREET SALISBURY, MA 01952 (978) 463-8980

FEBRUARY 6, 2018



CONCLUSIONS

The results of these calculations indicate the proposed stormwater management systems for the proposed development are capable of storing and treating the runoff for the 2-year, 10-year and 100-year storm events.

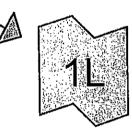
The peak flow rates in this analysis have been conservatively estimated for both the preand post-development conditions. Based on the results of the analyses described herein, the proposed development will not increase the runoff rate leaving the site. The proposed storm water management facilities shown on the Site Plan will produce no adverse storm water runoff impacts under the storms analyzed.

Condition	2-year	10-year	100-year
Pre-Development	2.2	5,2	13.0
Post Development	1.7	4.5	11.1

PRE-DEVELOPMENT DRAINAGE CALCULATIONS



Exist. Flow to DP



Design Point









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2/6/2018

Subcatchment Ex1: Exist. Flow to DP

Runoff 2.17 cfs @ 12.16 hrs, Volume= 8,452 cf, Depth> 0.92"

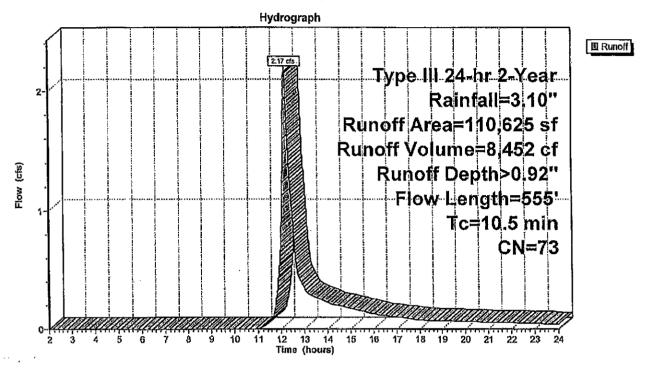
Runoff by SCS TR-20 method, UH=SCS, Time Span= 2.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 2-Year Rainfall=3.10"

_	Ar	ea (sf)	CN D	Description			
		2,964	98 E	Buildings			
		105		Brick Walk			
	2,633		90 (3ravel road	s, HSG C/I)	
	51,836			Woods, Fair, HSG C/D			
	5,480			Woods, Fair, HSG A			
	12,187			50-75% Grass cover, Fair, HSG A			
_	35,420					Fair, HSG C	
		110,625		Weighted A	_		
	1	07,556	_	Pervious Ar			
		3,069		mpervious	Area		
	T	1	Clana	Valaalhi	Cannaite	Description	
	Tc	Length	Slope		Capacity (cfs)	Description	
-	(min)	(feet)	(ft/ft)		(615)	Chart Flaur	
	5.4	95	0.0830	0.29		Sheet Flow, Grass: Short n= 0.150 P2= 3.10"	
	0.5	130	0.0830	4.64		Shallow Concentrated Flow,	
	0.5	130	0.0000	4,04		Unpayed Ky= 16.1 fps	
	0.7	60	0.0830	1.44		Shallow Concentrated Flow,	
	0.7	00	0.0000	11-4-1		Woodland Kv= 5.0 fps	
	0.5	40	0.0830	1.44		Shallow Concentrated Flow,	
	444		0.442			Woodland Kv= 5.0 fps	
	3.4	230	0.0500	1.12		Shallow Concentrated Flow,	
						Woodland Kv= 5.0 fps	
•	10.5	555	Total				

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Subcatchment Ex1: Exist. Flow to DP



2/6/2018

Link 1L: Design Point

Inflow Area =

110,625 sf, Inflow Depth > 0.92" for 2-Year event

Inflow

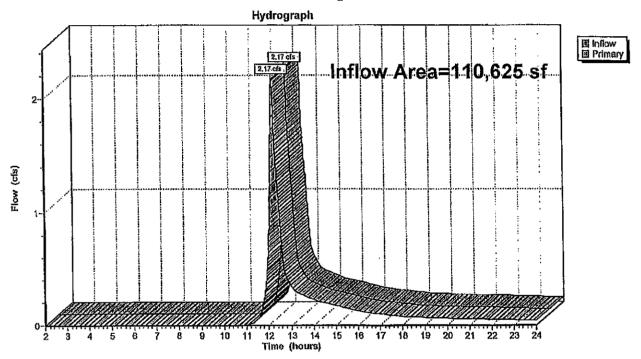
Primary

2.17 cfs @ 12.16 hrs, Volume= 2.17 cfs @ 12.16 hrs, Volume=

8,452 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 2.00-24.00 hrs, dt= 0.01 hrs

Link 1L: Design Point



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2/6/2018

Subcatchment Ex1: Exist. Flow to DP

Runoff

5.17 cfs @ 12.15 hrs, Volume=

18,833 cf, Depth> 2.04"

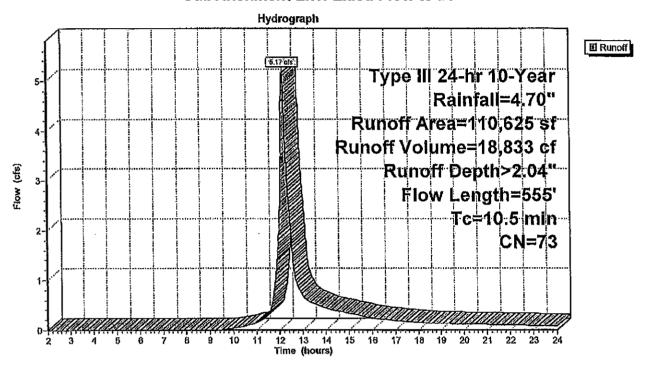
Runoff by SCS TR-20 method, UH=SCS, Time Span= 2.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10-Year Rainfall=4.70"

	Ar	ea (s f)	CN [Description		
		2,964	98 E	Buildings		
		105	98 E	3rick Walk		
		2,633		Gravel road		
	£	51,836		<i>N</i> oods, Faiı	•)
		5,480		Woods, Faiı		
		12,187				Fair, HSG A
		35,420				Fair, HSG C
		10,625		Weighted A		
	10)7,556		Pervious Ar		
		3,069		Impervious	Area	
			~1	N.C. 1 . W	O	Dunandaklar
	Tc	Length	Slope		Capacity	Description
<u> (mi</u>		(feet)	(ft/ft)	***************************************	(cfs)	OL _ 4 7!
5	5.4	95	0.0830	0.29		Sheet Flow,
.		400	0.0000	4.64		Grass: Short n= 0.150 P2= 3.10" Shallow Concentrated Flow,
ι).5	130	0.0830	4.64		Unpaved Kv= 16.1 fps
r	0.7	60	0.0830	1.44		Shallow Concentrated Flow,
), [00	0.0000	, 1		Woodland Kv= 5.0 fps
).5	40	0.0830	1.44		Shallow Concentrated Flow,
`	J. W	70	0.0000			Woodland Kv= 5.0 fps
5	3.4	230	0.0500	1.12		Shallow Concentrated Flow,
`	٠. ١	200		,		Woodland Kv= 5.0 fps
10	0.5	555	Total			

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Subcatchment Ex1: Exist. Flow to DP



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Link 1L: Design Point

Inflow Area =

110,625 sf, Inflow Depth > 2.04" for 10-Year event

Inflow

5.17 cfs @ 12.15 hrs, Volume= 5.17 cfs @ 12.15 hrs, Volume=

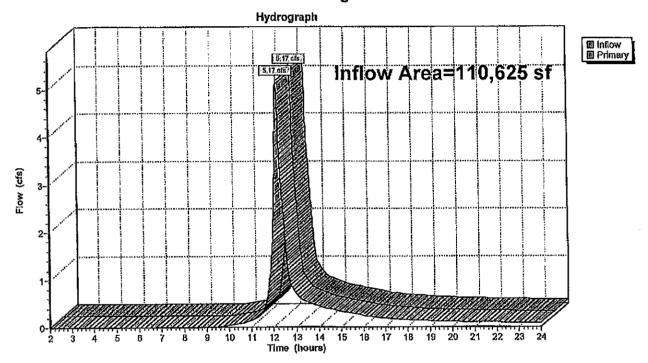
18.833 cf

Primary

18,833 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 2.00-24.00 hrs, dt= 0.01 hrs

Link 1L: Design Point



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Subcatchment Ex1: Exist. Flow to DP

Runoff

12.98 cfs @ 12.15 hrs, Volume=

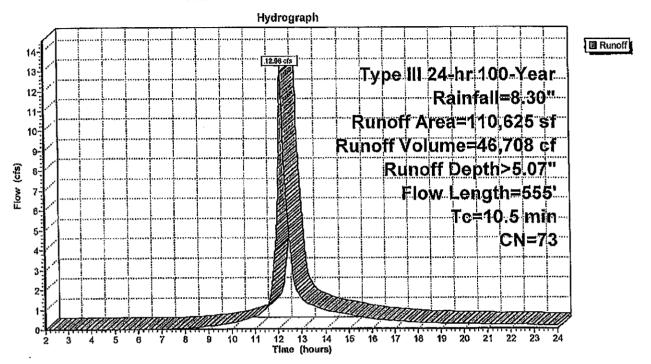
46,708 cf, Depth> 5.07"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 2.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 100-Year Rainfall=8.30"

	Ar	ea (sf)	CN D	escription						
•		2,964	98 B	Buildings						
		105	98 B	Brick Walk						
		2,633	90 G	iravel road	s, HSG C/E)				
		51,836			r, HSG C/D					
		5,480		Voods, Fai						
		12,187				Fair, HSG A				
,		<u> 35,420 </u>				Fair, HSG C				
		10,625		Veighted A		•				
	1	07,556		ervious Ar						
3,069 Impervious Area										
		Langth	Clana	Volosity	Canacity	Description				
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description				
	5.4	95	0.0830	0.29	(019)	Sheet Flow,				
	5,4	90	0.0030	0.20		Grass: Short n= 0.150 P2= 3.10"				
	0.5	130	0.0830	4.64		Shallow Concentrated Flow,				
	0.0	100	0,000	1.01		Unpaved Kv= 16.1 fps				
	0.7	60	0.0830	1.44	Shallow Concentrated Flow,					
						Woodland Kv= 5.0 fps				
	0.5	40	0.0830	1.44		Shallow Concentrated Flow,				
						Woodland Kv= 5.0 fps				
	3.4	230	0.0500	1.12		Shallow Concentrated Flow,				
						Woodland Kv= 5.0 fps				
	10.5	555	Total							

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Subcatchment Ex1: Exist. Flow to DP



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2/6/2018

Link 1L: Design Point

Inflow Area =

110,625 sf, inflow Depth > 5.07" for 100-Year event

Inflow

12.98 cfs @ 12.15 hrs, Volume=

46,708 cf

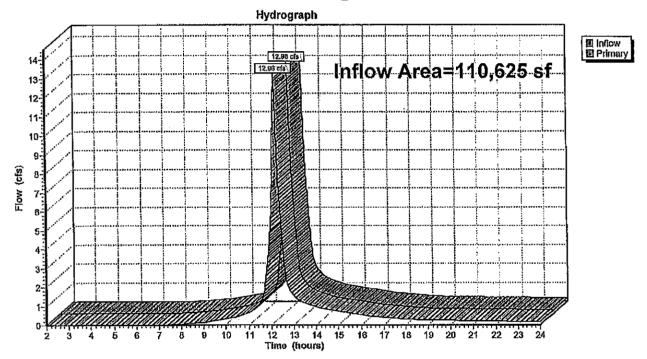
Primary

12.98 cfs @ 12.15 hrs, Volume=

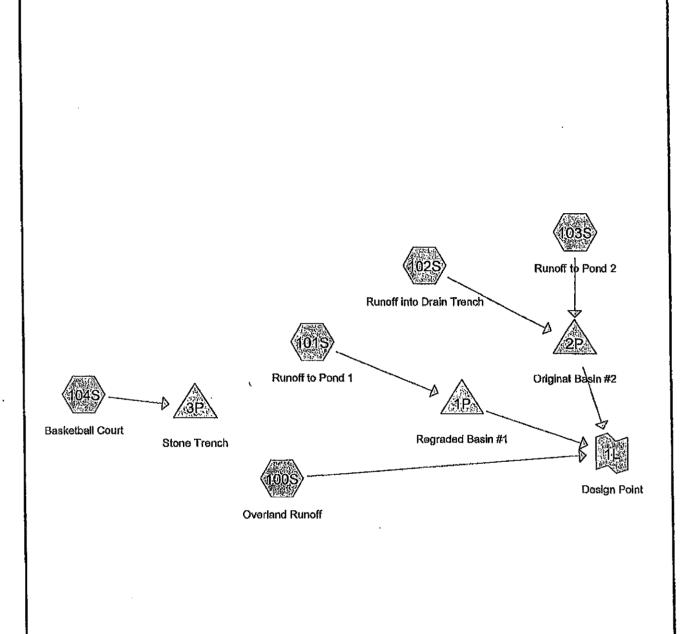
46,708 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 2.00-24.00 hrs, dt= 0.01 hrs

Link 1L: Design Point



POST-DEVELOPMENT DRAINAGE CALCULATIONS











2/6/2018

Subcatchment 100S: Overland Runoff

Runoff

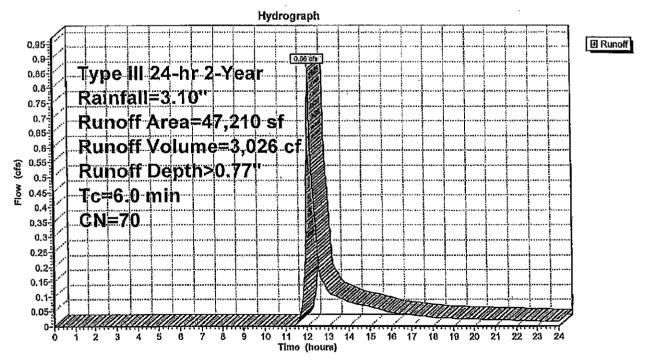
0.86 cfs @ 12.10 hrs, Volume=

3,026 cf, Depth> 0.77"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 2-Year Rainfall=3.10"

Are	a (sf)	CN	Description					
	4,982	49	50-75% Grass cover, Fair, HSG A					
	5,379	36	Woods, Fair, HSG A					
1	5,910	73	Woods, Fai	r, HSG C				
1	4,217	74	>75% Grass	s cover, Go	od, HSG C			
	6,722	98	mpervious Areas					
4	7,210	70	Weighted Average					
4	0,488		Pervious Ar	еа				
	6,722		Impervious	Area				
	Length	Slope	,	Capacity	Description			
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
6.0					Direct Entry,			

Subcatchment 100S: Overland Runoff



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2/6/2018

Subcatchment 101S: Runoff to Pond 1

Runoff

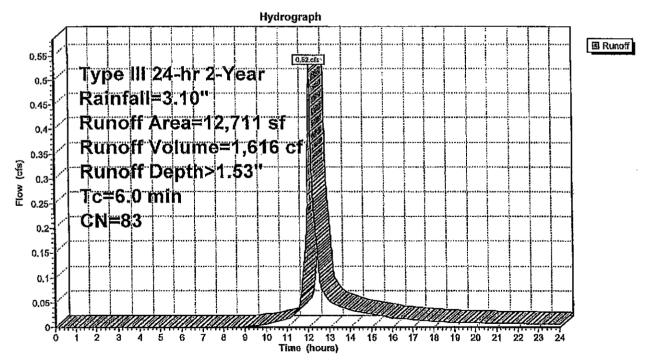
0.52 cfs @ 12.09 hrs, Volume=

1,616 cf, Depth> 1.53"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 2-Year Rainfall=3.10"

	Area (sf)	CN [Description							
	6,298	74 >	>75% Grass cover, Good, HSG C							
	1,413	73 \	Woods, Fair, HSG C							
	5,000	98 I	Impervious Areas							
	12,711 83 Weighted Average									
	7,711	-	Pervious Ar	ea						
	5,000	l	mpervious	Area						
		Olona	Voloaihu	Canadhi	Description					
T(Slope	•	Capacity	Description					
<u>(min</u>		(ft/ft)	(ft/sec)	(cfs)		<u></u>				
6.0)				Direct Entry,					

Subcatchment 101S: Runoff to Pond 1



HydroCAD® 8.00 s/n 002736 © 2006 HydroCAD Software Solutions LLC

2/6/2018

Pond 1P: Regraded Basin #1

Inflow Area = 12,711 sf, Inflow Depth > 1.53" for 2-Year event

Inflow = 0.52 cfs @ 12.09 hrs, Volume= 1,616 cf

Outflow = 0.29 cfs @ 12.22 hrs, Volume= 1,615 cf, Atten= 44%, Lag= 7.9 min

Discarded = 0.11 cfs @ 12.22 hrs, Volume= 1,339 cf

Primary = 0.18 cfs @ 12.22 hrs, Volume= 277 cf

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 56.50' @ 12.22 hrs Surf.Area= 569 sf Storage= 252 cf Flood Elev= 58.00' Surf.Area= 1,040 sf Storage= 1,444 cf

Plug-Flow detention time= 8.9 min calculated for 1,615 cf (100% of inflow) Center-of-Mass det. time= 8.6 min (842.3 - 833.7)

<u>Volume</u>	<u>Invert</u>	Avail.Sto	rage Storage	Description		
#1	56.00	1,44	44 cf Custom	Stage Data (Conic	c) Listed below (Red	calc)
Elevation (fee		urf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
56.0 57.0 58.0	00	440 715 1,040	0 572 872	0 572 1,444	440 728 1,069	
Device	Routing	Invert	Outlet Device	s		
#1 #2	Discarded Primary	0.00' 56,20'	6.0" x 15.0' l Outlet Invert=	kfiltration over Sul ong Culvert CPP, 56.00' S= 0.0133 rugated PE, smoot	, projecting, no head 3 '/'	dwall, Ke= 0.900

Discarded OutFlow Max=0.11 cfs @ 12.22 hrs HW=56.50' (Free Discharge)

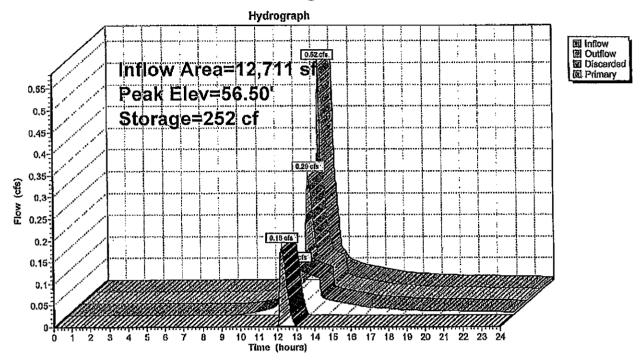
1=Exfiltration (Exfiltration Controls 0.11 cfs)

Primary OutFlow Max=0.18 cfs @ 12.22 hrs HW=56.50' (Free Discharge) —2=Culvert (Inlet Controls 0.18 cfs @ 1.47 fps)

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Pond 1P: Regraded Basin #1



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Subcatchment 102S: Runoff into Drain Trench

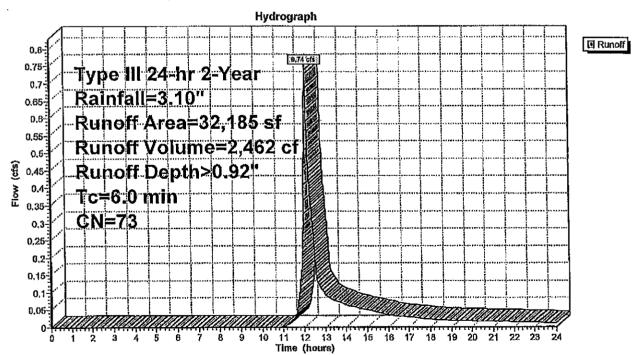
Runoff = 0.74 cfs @ 12.10 hrs, Volume=

2,462 cf, Depth> 0.92"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 2-Year Rainfall=3.10"

Ar	ea (sf)	CN	Description				
	7,276	49	50-75% Gra	ss cover, F	Fair, HSG A		
•	12,022	79	50-75% Gra	ss cover, F	Fair, HSG C		
	2,220	74	>75% Grass	s cover, Go	ood, HSG C		
	6,817	73	Woods, Fai	r, HSG C			
	3,850	98	Impervious Areas				
	32,185	73	73 Weighted Average				
	28,335		Pervious Ar	ea			
	3,850		Impervious	Area			
Tc (min)	Length (feet)	Slope (ft/ft	•	Capacity (cfs)			
6.0	(1000)	11016	(15000)	(0.0)	Direct Entry,		

Subcatchment 102S: Runoff into Drain Trench



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Subcatchment 103S: Runoff to Pond 2

Runoff

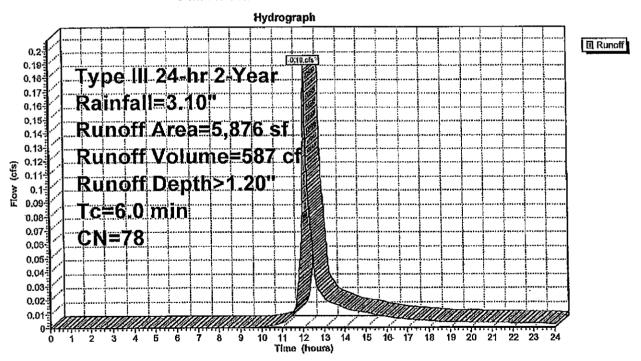
0.19 cfs @ 12.09 hrs, Volume=

587 cf, Depth> 1.20"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 2-Year Rainfall=3.10"

	Αı	ea (sf)	CN	Description						
		4,776	74	>75% Grass cover, Good, HSG C						
		1,100	98	Impervious Areas						
		5,876 4,776 1,100	78	Weighted Average Pervious Area Impervious Area						
(Tc min)	Length (feet)	Slop (ft/ft	•	Capacity (cfs)	Description				
	6.0					Direct Entry,				

Subcatchment 103S: Runoff to Pond 2



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Pond 2P: Original Basin #2

38,061 sf, Inflow Depth > 0.96" for 2-Year event Inflow Area =

3.049 cf 0.93 cfs @ 12.10 hrs, Volume= Inflow

2.994 cf. Atten= 17%, Lag= 3.3 min 0.77 cfs @ 12.15 hrs, Volume= Outflow ==

0.77 cfs @ 12.15 hrs, Volume= 2.994 cf Primary

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 50.75' @ 12.15 hrs Surf.Area= 381 sf Storage= 219 cf Flood Elev= 52.20' Surf.Area= 740 sf Storage= 1,029 cf

Plug-Flow detention time= 16.6 min calculated for 2,994 cf (98% of inflow)

Center-of-Mass det. time= 6.5 min (869.9 - 863.4)

Volume	Inv	ert Avail.Sto	rage Storage	Description		
#1	50,0	1,0	29 cf Custom	Stage Data (Conic	c) Listed below (Red	calc)
Elevation (fee		Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
50.0 51.0 52.3	00	214 448 740	0 324 706	0 324 1,029	214 456 766	
Device	Routing	Invert	Outlet Device	s		
#1	Primary	48.30'	Outlet Invert=	long Culvert CPR 46.50' S= 0.0305 rrugated PE, smoot		dwall, Ke= 0.500
#2 #3	Device			3' H Vert. Orifice/G Orifice/Grate Lin	Frate C≃ 0.600	>= 0 600

Primary OutFlow Max=0.77 cfs @ 12.15 hrs HW=50.75' (Free Discharge)

-1=Culvert (Passes 0.77 cfs of 5.28 cfs potential flow)

-2=Orifice/Grate (Orifice Controls 0.77 cfs @ 2.95 fps)

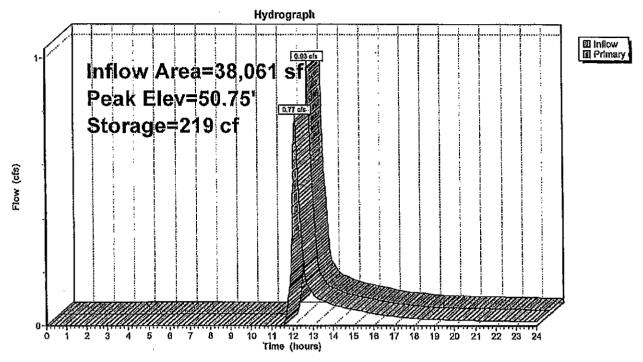
☐3=Orifice/Grate (Controls 0.00 cfs)

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Pond 2P: Original Basin #2



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Link 1L: Design Point

Inflow Area =

97,982 sf, Inflow Depth > 0.77" for 2-Year event

Inflow =

1.70 cfs @ 12.13 hrs, Volume=

6,297 cf

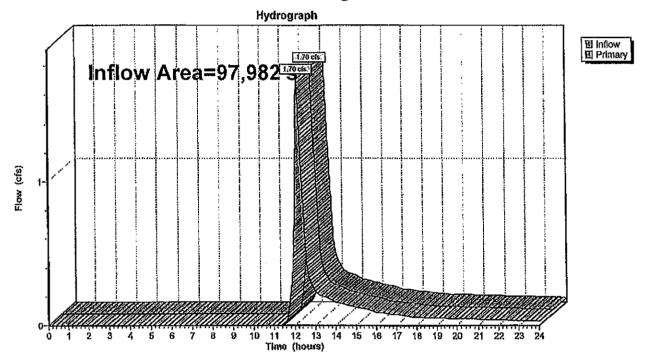
Primary =

1.70 cfs @ 12.13 hrs, Volume=

6,297 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs

Link 1L: Design Point



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Subcatchment 104S: Basketball Court

Runoff

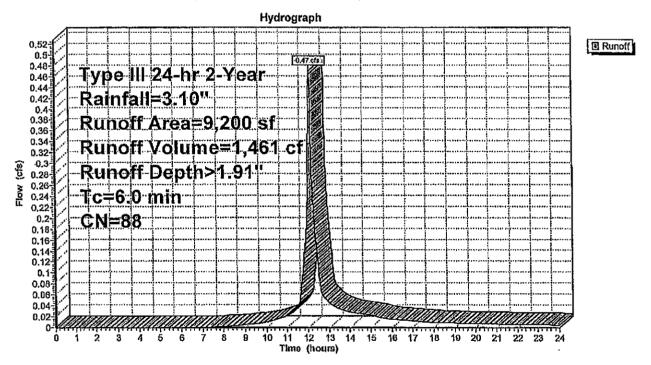
0.47 cfs @ 12.09 hrs, Volume=

1,461 cf, Depth> 1.91"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 2-Year Rainfall=3.10"

	Area (sf)	CN	Description							
	5,200	98	Impervious Area							
	4,000	74	>75% Grass cover, Good, HSG C							
<u> </u>	9,200	88	Weighted Average Pervious Area							
	4,000									
	5,200		Impervious	Area						
Ţ	₩	Slope	•	Capacity	Description					
(min	i) (feet)	(ft/ft) (ft/sec)	(cfs)						
6.4	0				Direct Entry,					

Subcatchment 104S: Basketball Court



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Pond 3P: Stone Trench

Inflow Area = 9,200 sf, Inflow Depth > 1.91" for 2-Year event Inflow = 0.47 cfs @ 12.09 hrs, Volume= 1,461 cf

Outflow = 0.25 cfs @ 12.01 hrs, Volume= 1,460 cf, Atten= 48%, Lag= 0.0 min

Discarded = 0.25 cfs @ 12.01 hrs, Volume= 1,460 cf

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 0.23' @ 12.23 hrs Surf.Area= 1,280 sf Storage= 119 cf Flood Elev= 3.00' Surf.Area= 1,280 sf Storage= 1,536 cf

Plug-Flow detention time= 2.5 min calculated for 1,460 cf (100% of inflow) Center-of-Mass det. time= 2.4 min (818.1 - 815.7)

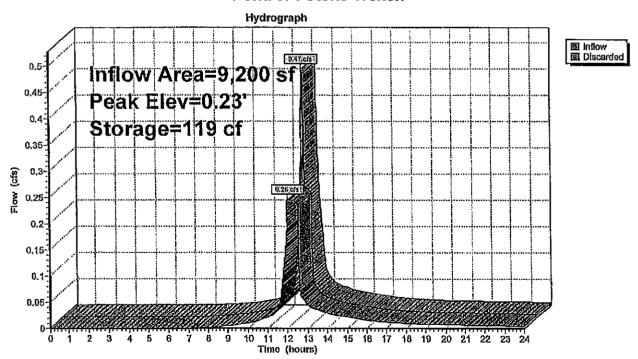
Volume .	Invert	Avail,Storage	Storage Description
#1	0.00'	1,536 cf	16.00'W x 80.00'L x 3.00'H Prismatoid
			3,840 cf Overall x 40,0% Voids

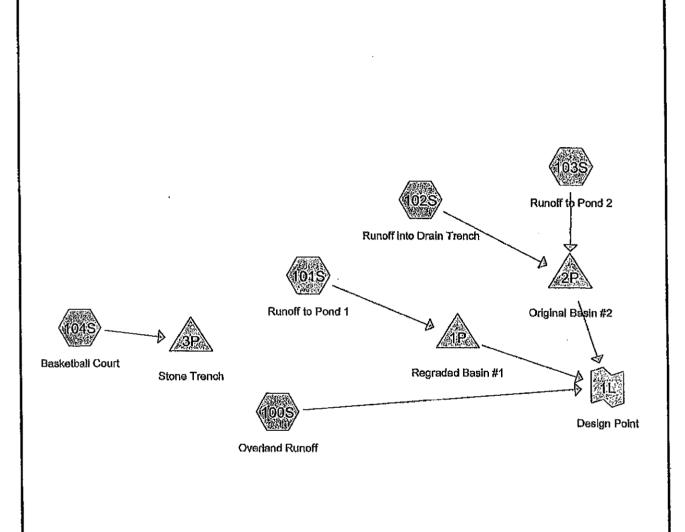
Device Routing Invert Outlet Devices

#1 Discarded 0.00' 8.270 in/hr Exfiltration over Surface area

Discarded OutFlow Max=0.25 cfs @ 12.01 hrs HW=0.03' (Free Discharge)
—1=Exfiltration (Exfiltration Controls 0.25 cfs)

Pond 3P: Stone Trench













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Type III 24-hr 10-Year Rainfall=4.70"

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Subcatchment 100S: Overland Runoff

Runoff

2.25 cfs @ 12.09 hrs, Volume=

7,137 cf, Depth> 1.81"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10-Year Rainfall=4.70"

	rea (sf)	CN	Description						
	4,982	49	50-75% Gra	ess cover, F	Water the state of				
	5,379		Woods, Fai						
	15,910	73	Woods, Fair, HSG C						
	14,217	74	>75% Grass cover, Good, HSG C						
	6,722	98	Impervious Areas						
	47,210 70 Weighted Average								
	40,488		Pervious Ar	ea					
	6,722		Impervious	Area					
Tc		Slope		Capacity	Description				
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)					
6.0					Direct Entry,				

Subcatchment 101S: Runoff to Pond 1

Runoff

0.99 cfs @ 12.09 hrs, Volume=

3,073 cf, Depth> 2.90"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10-Year Rainfall=4.70"

Α	rea (sf)	CN I	Description						
	6,298	74 :	>75% Gras						
	1,413	73	Noods, Fai						
	5,000	98	Impervious Areas						
	12,711	83	83 Weighted Average						
	7,711		Pervious A	ea -					
	5,000	1	mpervious	Area					
Tc (min)	Length (feet)	Slope (fl/ft)	•	Capacity (cfs)	Description				
6.0				•	Direct Entry,				

Subcatchment 102S: Runoff into Drain Trench

Runoff

1.76 cfs @ 12.09 hrs, Volume=

5,485 cf, Depth> 2.05"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10-Year Rainfall=4.70"

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<i>P</i>	∖rea (sf)	CN	<u>CN</u> Description					
	7,276	49	50-75% Grass cover, Fair, HSG A					
	12,022	79	50-75% Gra	ass cover, F	fair, HSG C			
	2,220	74	>75% Grass	s cover, Go	od, HSG C			
	6,817	73	Woods, Fai	r, HSG C				
	3,850	98	Impervious	Areas				
	32,185	73	Weighted A	verage	<u> </u>			
	28,335		Pervious Ar	ea				
	3,850		Impervious	Area				
To	Length	Slope	e Velocity	Capacity	Description			
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)	·			
6.0					Direct Entry.			

Subcatchment 103S: Runoff to Pond 2

Runoff

0.39 cfs @ 12.09 hrs, Volume=

1,203 cf, Depth> 2.46"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10-Year Rainfall=4.70"

	Aı	rea (sf)	CN	Description			
_		4,776 1,100		>75% Gras Impervious	-	od, HSG C	
		5,876 4,776 1,100		Weighted A Pervious Ai Impervious	ea		
	Tc (min)	Length (feet)	Slope (ft/ft		Capacity (cfs)	Description	
-	6.0					Direct Entry.	

Subcatchment 104S: Basketball Court

Runoff

0.82 cfs @ 12.09 hrs, Volume=

2,592 cf, Depth> 3.38"

Runoff by SCS TR-20 method, UH=SCS, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Type III 24-hr 10-Year Rainfall=4.70"

	Area (sf)	CN	Description
	5,200	98	Impervious Area
	4,000	74	>75% Grass cover, Good, HSG C
-	9,200	88	Weighted Average
	4,000		Pervious Area
	5,200		Impervious Area

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Type III 24-hr 10-Year Rainfall=4.70"

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Tc	Length	Slope	Velocity	Capacity	Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
6.0					Direct Entry,	

Pond 1P: Regraded Basin #1

Inflow Area =

12,711 sf, Inflow Depth > 2,90" for 10-Year event

Inflow

0.99 cfs @ 12.09 hrs, Volume= 3.073 cf

=

Outflow

0.60 cfs @ 12.19 hrs, Volume=

3,072 cf. Atten= 39%, Lag= 6.3 min

Discarded =

0.13 cfs @ 12.19 hrs, Volume=

2.119 cf

Primary

0.47 cfs @ 12.19 hrs, Volume=

953 cf

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 56.85' @ 12.19 hrs Surf.Area= 670 sf Storage= 469 cf Flood Elev= 58.00' Surf.Area= 1,040 sf Storage= 1,444 cf

Plug-Flow detention time= 9.5 min calculated for 3,072 cf (100% of inflow) Center-of-Mass det. time= 9.3 mln (824.6 - 815.3)

Volume	Invert	Avail.Sto	rage Storage	Description		
#1	56.00	1,44	44 cf Custom	Stage Data (Conic	c) Listed below (F	Recalc)
Elevation (fee		ırf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
56.0 57.0 58.0	00	440 715 1,040	0 572 872	0 572 1,444	440 728 1,069	
Device	Routing	Invert	Outlet Device	S		
#1 #2	Discarded Primary	0.00' 56.20'	6.0" x 15.0' le Outlet Invert=	cfiltration over Sui ong Culvert CPP, 56.00' S= 0.0133 rugated PE, smoot	, projecting, no he 3 '/'	eadwall, Ke= 0.900

Discarded OutFlow Max=0.13 cfs @ 12.19 hrs HW=56.85' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.13 cfs)

Primary OutFlow Max=0.47 cfs @ 12.19 hrs HW=56.85' (Free Discharge) 2=Culvert (inlet Controls 0.47 cfs @ 2.41 fps)

Pond 2P: Original Basin #2

38,061 sf, Inflow Depth > 2.11" for 10-Year event Inflow Area =

Inflow 2.14 cfs @ 12.09 hrs, Volume= 6.688 cf

Outflow 1.93 cfs @ 12.13 hrs, Volume= 6.628 cf, Atten= 10%, Lag= 2.4 min =

1.93 cfs @ 12.13 hrs. Volume= Primary 6.628 cf

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs. dt= 0.01 hrs Peak Elev= 51.37' @ 12.13 hrs Surf.Area= 531 sf Storage= 506 cf Flood Elev= 52.20' Surf.Area= 740 sf Storage= 1,029 cf

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Plug-Flow detention time= 10.7 min calculated for 6,628 cf (99% of inflow)

Center-of-Mass det. time= 5.4 mln (845.4 - 840.0)

<u>Volume</u>	Inve	ert Avail.Sto	rage Storage I	Description		
#1	50.0	00' 1,02	29 cf Custom	Stage Data (Conic	c) Listed below (Re	calc)
Elevatio	* .	Surf,Area (sq-ft)	inc.Store (cubic-feet)	Cum.Store (cubic-feet)	Wet.Area (sq-ft)	
50.0 51.0 52.2	00	214 448 740	0 324 706	0 324 1,029	214 456 766	
Device	Routing	Invert	Outlet Devices	S		
#1	Primary	48.30'	Outlet Invert=	long Culvert CPF 46.50' S= 0.0305 ugated PE, smoot	7' Cc= 0.900	adwall, Ke= 0.500
#2 #3	Device 1 Device 1			' H Vert. Orifice/G prifice/Grate Lim	irate C= 0.600 nited to weir flow	C= 0.600

Primary OutFlow Max=1.93 cfs @ 12.13 hrs HW=51.37' (Free Discharge)

-1=Culvert (Passes 1.93 cfs of 6.06 cfs potential flow)

-2=Orifice/Grate (Orifice Controls 1.26 cfs @ 4.83 fps)

-3=Orifice/Grate (Weir Controls 0.67 cfs @ 1.36 fps)

Pond 3P: Stone Trench

Inflow Area = 9,200 sf, Inflow Depth > 3.38" for 10-Year event

Inflow = 0.82 cfs @ 12.09 hrs, Volume= 2,592 cf

Outflow = 0.25 cfs @ 11.85 hrs, Volume= 2,592 cf, Atten= 70%, Lag= 0.0 min

Discarded = 0.25 cfs @ 11.85 hrs, Volume= 2,592 cf

Routing by Stor-Ind method, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs Peak Elev= 0.88' @ 12.42 hrs Surf.Area= 1,280 sf Storage= 449 cf

Flood Elev= 3.00' Surf.Area= 1.280 sf Storage= 1.536 cf

Plug-Flow detention time= 9.0 min calculated for 2,592 cf (100% of inflow)

Center-of-Mass det. tlme= 8.9 min (808.4 - 799.5)

Volume	Invert	Avail.Stora	ge Storage Description
#1	0.00'	1,536	The state of the s
			3,840 cf Overall x 40.0% Voids
Device	Routing	Invert	Outlet Devices
#1	Discarded	0.00'	8.270 in/hr Exfiltration over Surface area

Discarded OutFlow Max=0.25 cfs @ 11.85 hrs HW=0.03' (Free Discharge) 1=Exfiltration (Exfiltration Controls 0.25 cfs)

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Type III 24-hr 10-Year Rainfall=4.70"

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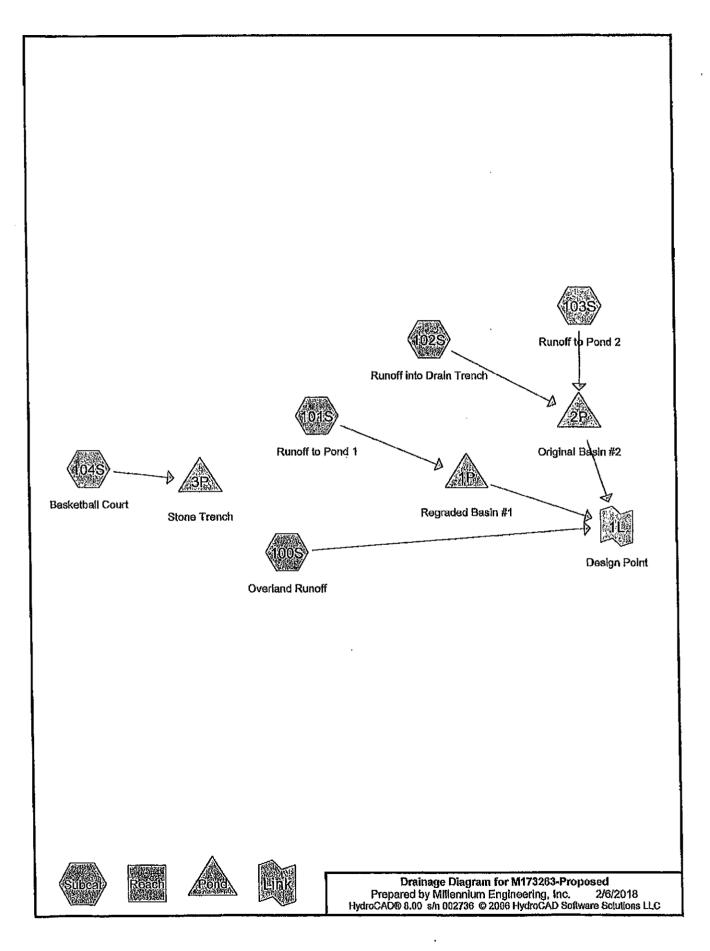
Link 1L: Design Point

97,982 sf, Inflow Depth > 1.80" for 10-Year event 4.48 cfs @ 12.12 hrs, Volume= 14,718 cf 4.48 cfs @ 12.12 hrs, Volume= 14,718 cf, Atte Inflow Area =

Inflow

Primary 14,718 cf, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-24.00 hrs, dt= 0.01 hrs



Area Listing (selected nodes)

Area (sq-ft)	<u>CN</u>	Description (subcats)
5,379	36	Woods, Fair, HSG A (100S)
12,258	49	50-75% Grass cover, Fair, HSG A (100S,102S)
24,140	73	Woods, Fair, HSG C (100S,101S,102S)
31,511	74	>75% Grass cover, Good, HSG C (1008,1018,1028,1038,1048)
12,022	79	50-75% Grass cover, Fair, HSG C (102S)
5,200	98	Impervious Area (104S)
16,672	98	Impervious Areas (100S,101S,102S,103S)
107,182		

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Time span=0.00-24.00 hrs, dt=0.01 hrs, 2401 points
Runoff by SCS TR-20 method, UH=SCS
Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 100S: Overland Runoff

Runoff Area=47,210 sf Runoff Depth>4.72"

Tc=6.0 min CN=70 Runoff=6.00 cfs 18,561 cf

Subcatchment 101S: Runoff to Pond 1

Runoff Area=12,711 sf Runoff Depth>6.26"

Tc=6.0 min CN=83 Runoff=2.08 cfs 6,629 cf

Subcatchment 102S: Runoff into Drain Trench

Runoff Area=32,185 sf Runoff Depth>5.07"

Tc=6.0 min CN=73 Runoff=4.39 cfs 13,601 cf

Subcatchment 103S: Runoff to Pond 2

Runoff Area=5,876 sf Runoff Depth>5.66"

Tc=6.0 min CN=78 Runoff=0.89 cfs 2,773 cf

Subcatchment 104S: Basketball Court

Runoff Area=9,200 sf Runoff Depth>6.86"

Tc=6.0 min CN=88 Runoff=1.61 cfs 5,256 cf

Pond 1P: Regraded Basin #1

Peak Elev=57.68' Storage=1,134 cf Inflow=2.08 cfs 6,629 cf

Discarded=0.18 cfs 3,684 cf Primary=0.83 cfs 2,943 cf Outflow=1.01 cfs 6,627 cf

Pond 2P: Original Basin #2

Peak Elev=52.06' Storage=925 cf Inflow=5.28 cfs 16,374 cf

Outflow=4.57 cfs 16,307 cf

Pond 3P: Stone Trench

Peak Elev=2,95' Storage=1,508 cf Inflow=1.61 cfs 5,256 cf

Outflow=0.25 cfs 5,255 cf

Link 1L: Design Point

Inflow=11.08 cfs 37,810 cf

Primary=11.08 cfs 37,810 cf

Total Runoff Area = 107,182 sf Runoff Volume = 46,820 cf Average Runoff Depth = 5.24"
79.59% Pervious Area = 85,310 sf 20.41% Impervious Area = 21,872 sf

WATERSHED PLANS

