Town of Newburyport Attn: Rob Ciampitti

RE:

12 Harrison St Newburyport MA 01950

Dear Rob,

We are scheduled for ZBA meeting May 26 for the abovementioned address. Attached are the revised plans that were accepted by the NHC. Let me know if you require any other information.

Thanks Mike Gray

5-6-2020

HARRISON ST

12 Harrison St. Newburyport, MA





Family and Type	Rough Height	Rough Width
r army and Type	rioigni	VVIGUI
Fixed: 16" x 24" 2		
Fixed: 16" x 24" 3		
Fixed: 16" x 24" 4		
Fixed: 16" x 24" 5		
Fixed: 16" x 24" 6		
Window-Double-Hung black front large: 24" x 42"	5' - 0 1/2"	2' - 7 1/2'
Window-Double-Hung black front large: 24" x 42" 8	4' - 0 1/2"	2' - 7 1/2'
Window-Double-Hung black front large: 24" x 42" 9	4' - 6 1/2"	2' - 7 1/2"
Window-Double-Hung black: 24" x 42"	5' - 0 1/2"	2' - 7 1/2'
Window-Double-Hung black: 24" x 42" 8	4' - 0 1/2"	2' - 7 1/2'
Window-Double-Hung black: 24" x 42" 9	4' - 6 1/2"	2' - 7 1/2'
Window-Double-Hung black: 24" x 42" FRONT WINDOW	5' - 8 1/2"	2' - 7 1/2'
Window-Double-Hung: 24" x 42"	5' - 0 1/2"	2' - 7 1/2'
Window-Double-Hung: 24" x 42" 2	5' - 0 1/2"	2' - 0 1/2'
Window-Double-Hung: 24" x 42" 4	2' - 2 1/2"	3' - 2 1/2"
M/: 1 D 11 11 04"	41 4 4 /011	01 7 4 /01

Window-Double-Hung: 24" 4' - 1 1/2" 2' - 7 1/2" x 42" 5

Window-Double-Hung: 24" 3' - 0 1/2" 2' - 3 1/2" x 42" 6

Window-Double-Hung: 24" 4' - 0 1/2" 2' - 7 1/2" x 42" 8

Window Schedule

Sheet List		
Sheet Number	Sheet Name	
A0.0	Cover	
A0.1	Locus Map & Schedules	
A1.5	Details	
A1.1	Existing W/ Demolition Plans	
A1.2	Proposed Floor Plans & Roof Plan	
A1.3	Proposed Elevations	
A0.2	Existing Elevations	
A1.4	Sections & Framing Plan	
A1.7	Renderings	
A1.6	Roof Details	



HARRISON ST

12 Harrison St. Newburyport, MA



ARCHITECT:

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DRAWING TITLE

Locus Map & Schedules

Revision Schedule Number Date Revision Description

SCALE

DATE 5-6-2020

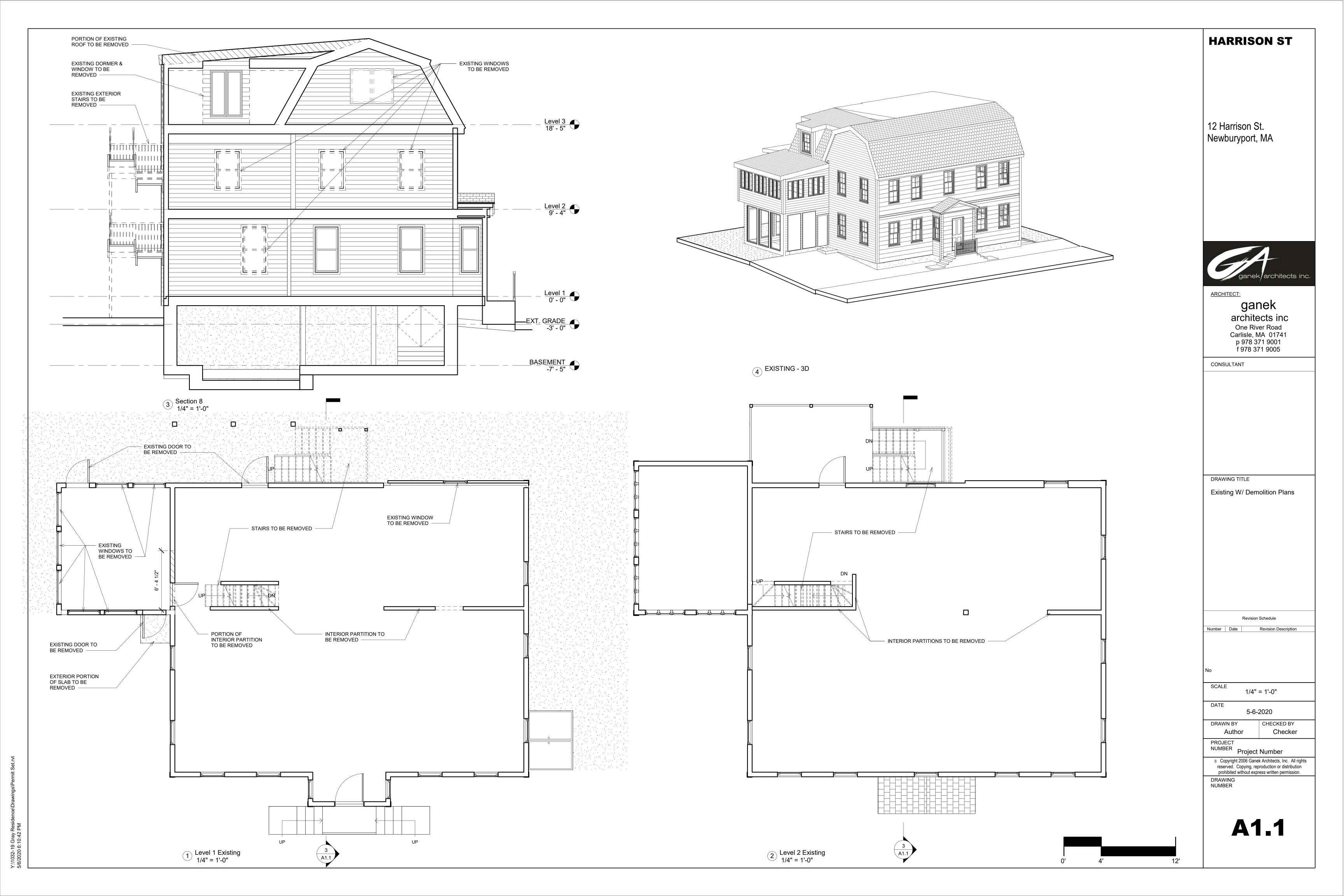
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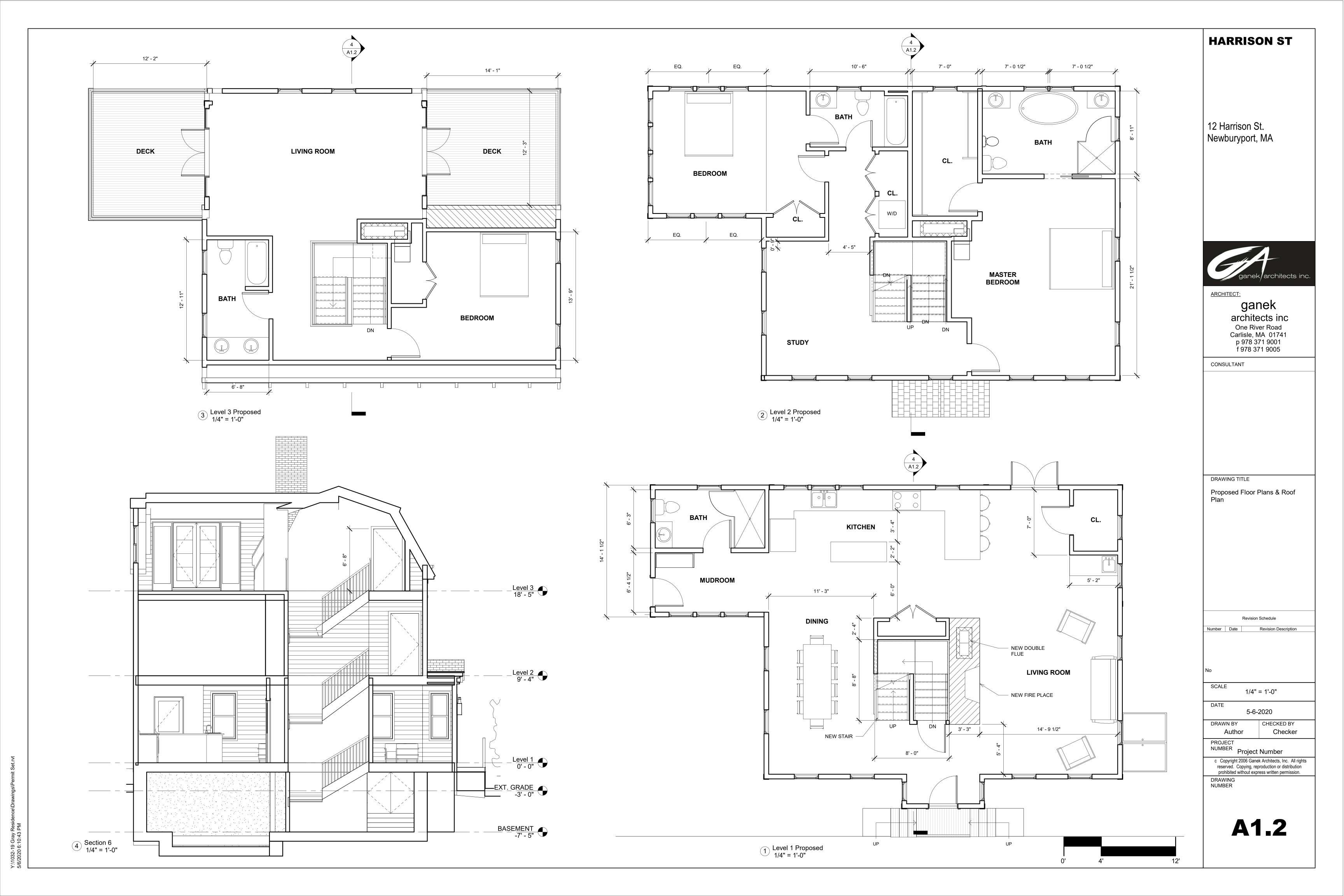
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Sections & Framing Plan

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5-6-2020

5-0-2020

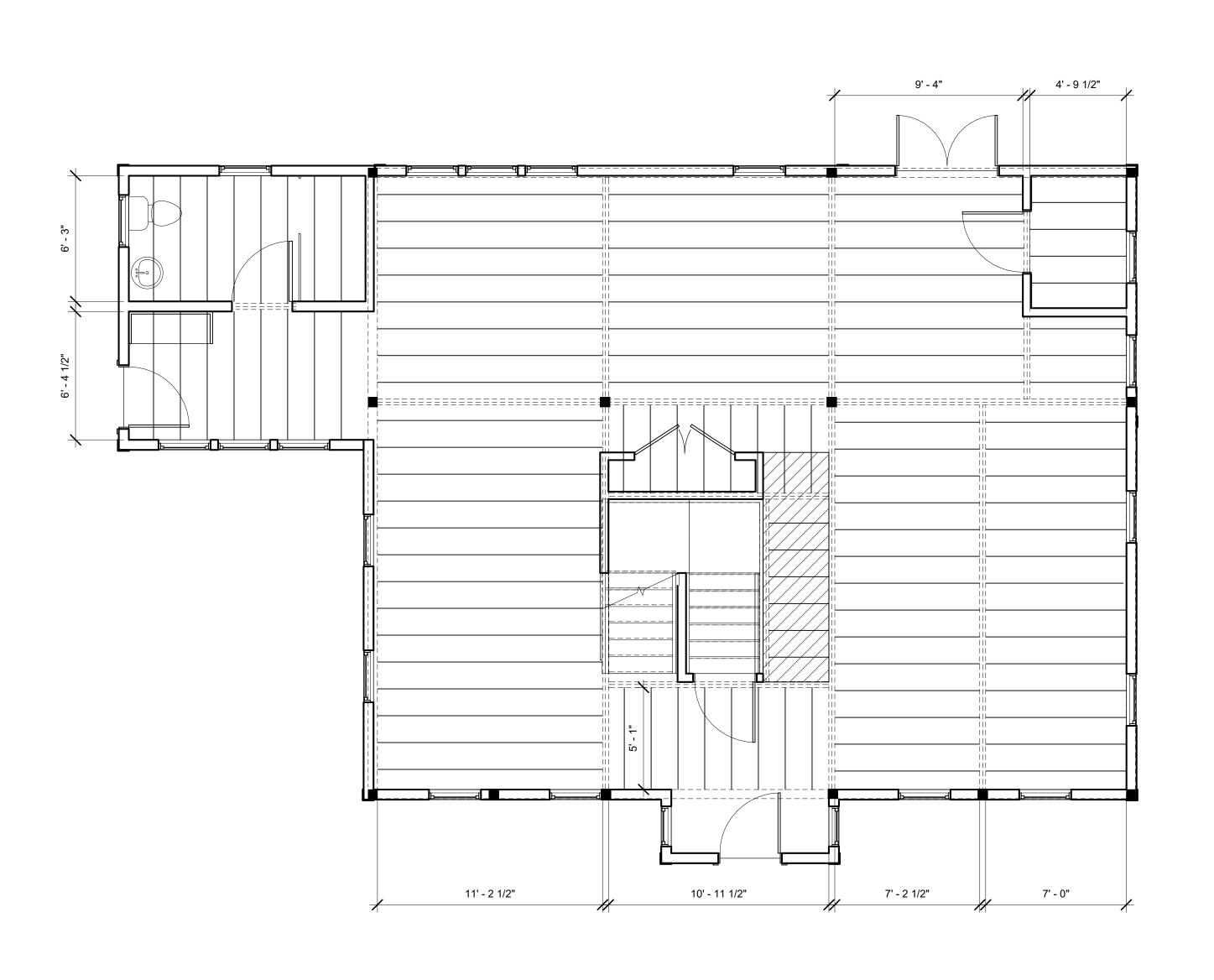
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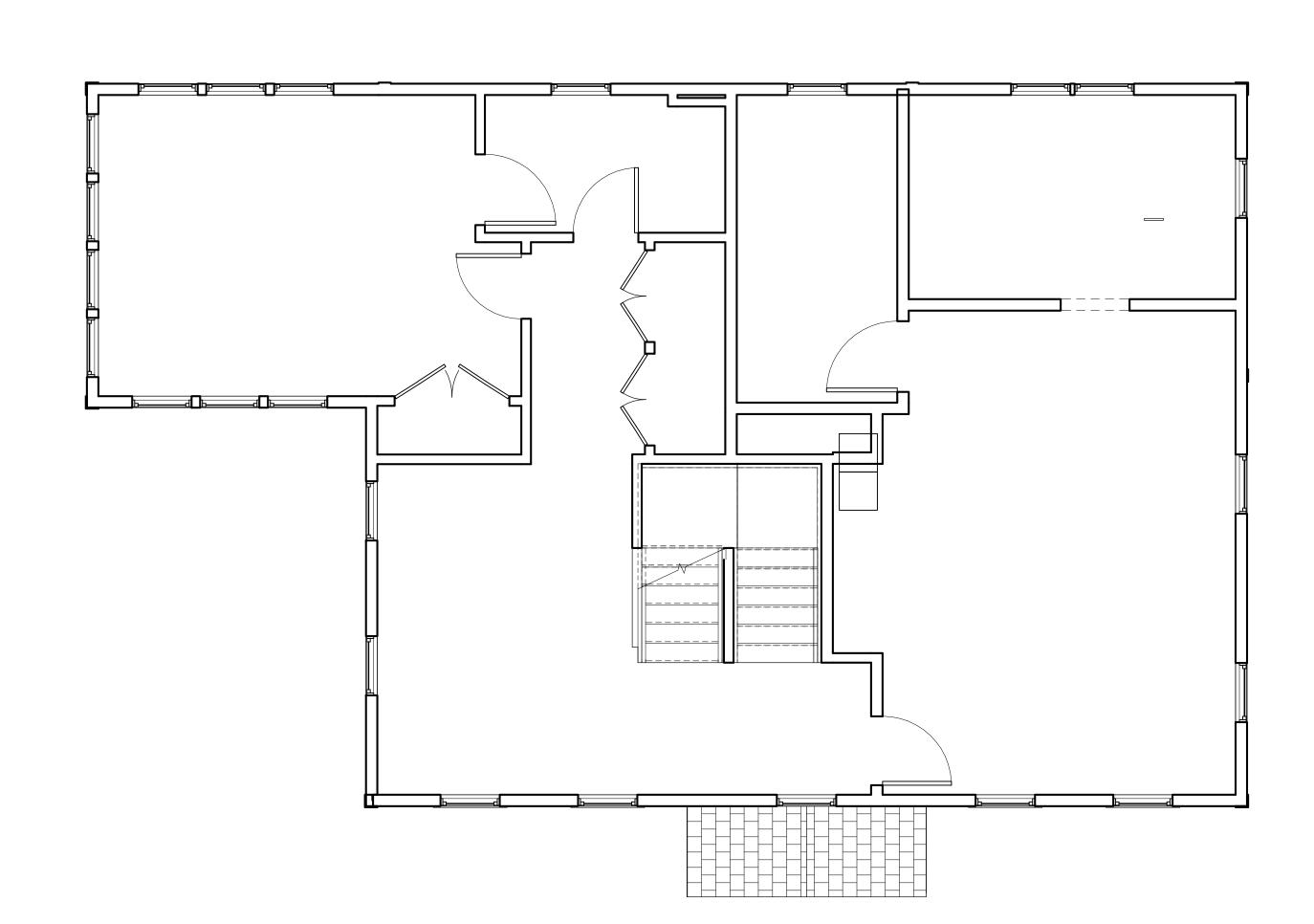
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A1.4







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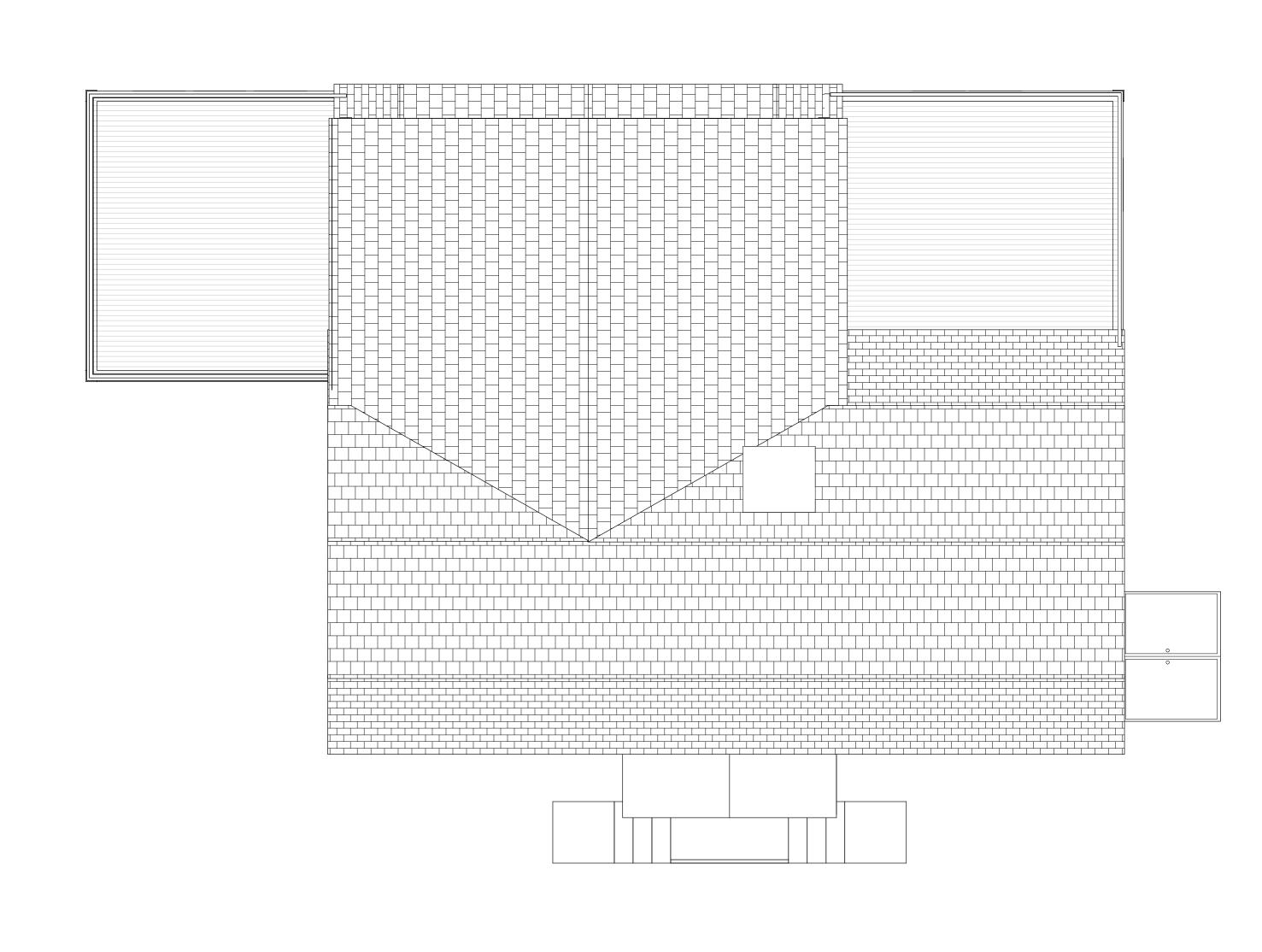
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Roof Details

Revision Schedule Number Date Revision Description

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A1.6



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DRAWING TITLE

Renderings

Revision Schedule

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A1.7

1 Exterior Perspective



GENERAL CONDITIONS

- 1. G. C. MUST BUILD EXACTLY WHAT IS SHOWN ON STRUCTURAL DRAWINGS. ANY PROPOSED DEPARTURES FROM WHAT IS INDICATED MUST BE REVIEWED WITH THE ENGINEER PRIOR TO CONSTRUCTION. ALL UNAUTHORIZED CHANGES TO THE APPROVED DRAWINGS MUST BE REMOVED AND REPLACED AT THE CONTRACTOR'S EXPENSE.
- 2. THE CONTRACTOR SHALL CAREFULLY VERIFY ALL DIMENSIONS AND CONDITIONS SHOWN ON DRAWINGS PRIOR TO COMMENCEMENT OF THE WORK, AND SHALL NOTIFY THE ENGINEER IMMEDIATELY OF ANY DISCREPANCIES BETWEEN ENGINEERING AND ARCHITECTURAL DOCUMENTS.
- 3. THE CONTRACTOR IS RESPONSIBLE FOR ALL MEANS AND METHODS OF TEMPORARY SHORING, BRACING, OR OTHERWISE PROTECTING ANY PORTION OF THE STRUCTURE, SITE AND UTILITIES FROM DAMAGE DURING CONSTRUCTION. THE ENGINEER IS SPECIFYING THE FINISHED CONDITION ONLY, WITHOUT ASSUMING KNOWLEDGE NOR RESPONSIBILITY FOR HOW THE CONTRACTOR WILL ACHIEVE THIS RESULT.
- 4. FOR RENOVATION WORK STRUCTURAL DRAWINGS PRODUCED WITH ASSUMPTIONS MADE REGARDING EXISTING CONDITIONS. IF CONTRACTOR FINDS EXISTING CONDITIONS NOT AS ASSUMED CONTACT ENGINEER IMMEDIATELY. REVISIONS TO THE STRUCTURAL FRAMING MAY BE REQUIRED.
- 5. FOR EXACT LOCATIONS OF FLOOR AND ROOF OPENINGS, POSTS, ETC., SEE ARCHITECTURAL DRAWINGS.

FOUNDATIONS

- 1. WHERE FOUNDATIONS ARE EXISTING, DESIGN HAS BEEN COMPLETED ASSUMING FOUNDATIONS ARE SUITABLE TO SUPPORT PROPOSED RENOVATION. CONTRACTOR RESPONSIBLE FOR VERIFYING THAT THE EXISTING FOUNDATION CONFORMS TO BUILDING CODE REQUIREMENTS AND REPORT FOOTING CONDITIONS TO ENGINEER FOR VERIFICATION.
- 2. EXCAVATE TO LINES AND GRADES REQUIRED TO PROPERLY INSTALL THE FOUNDATIONS ON INORGANIC, UNDISTURBED SOIL OR CONTROLLED STRUCTURAL BACKFILL AS REQUIRED BY THE ARCHITECT. ALL EXCAVATIONS SHALL BE DRY BEFORE PLACING ANY CONCRETE.
- 3. EXTERIOR FOOTINGS SHALL BE PLACED ON APPROVED SOIL AT A MINIMUM DEPTH OF 4 FEET, OR AS MODIFIED BY THE STRUCTURAL ENGINEER, BELOW THE LOWEST ADJACENT GROUND EXPOSED TO FREEZING. ANY ADJUSTMENT OF FOOTING ELEVATIONS DUE TO FIELD CONDITIONS MUST HAVE THE APPROVAL OF THE ARCHITECT.
- 4. SOIL BEARING CAPACITY: FOOTINGS MUST BE PLACED ON SOIL WITH A MINIMUM BEARING CAPACITY OF 4000 POUNDS PER SQUARE FOOT.
- BACKFILL BELOW FOOTINGS AND SLABS SHALL BE MADE WITH APPROVED GRANULAR MATERIALS PLACED IN 6" LAYERS. LAYERS SHALL BE COMPACTED TO 96% DENSITY AT OPTIMUM MOISTURE CONTENT, AS DEFINED BY ASTM D1557.
- 6. BACKFILLING AGAINST WALLS OR PIERS MAY ONLY BE DONE AFTER WALLS OR PIERS ARE BRACED TO PREVENT MOVEMENT. FOR WOOD FRAMED RESIDENTIAL CONSTRUCTION, NO BACKFILLING OF WALLS MAY TAKE PLACE UNTIL THE FIRST FLOOR DECK HAS BEEN FRAMED AND SHEATHED, UNLESS WRITTEN APPROVAL IS GIVEN BY THE ARCHITECT OR ENGINEER.
- 7. PROVIDE FOUNDATION DRAINAGE, WATERPROOFING/DAMP-PROOFING, AND FOUNDATION WALL INSULATION AS INDICATED ON THE ARCHITECTURAL DRAWINGS.

CONCRETE

WIRE FABRIC - ASTM A185.

- 1. ALL CONCRETE WORK SHALL BE PERFORMED IN CONFORMANCE WITH THE LATEST EDITION OF
- ACI-318, "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE". 2. CONCRETE SHALL ACHIEVE A MINIMUM 28 DAY DESIGN STRENGTH AS FOLLOWS:FOOTINGS, WALLS, INTERIOR SLABS-ON-GRADE, AND OTHER CONCRETE NOT OTHERWISE SPECIFIED - 3000 PSI. EXTERIOR SLABS EXPOSED TO WEATHER - 4000 PSI.
- 3. SLUMP AT THE POINT OF DISCHARGE FROM THE READY-MIX TRUCK SHALL BE 3-5". 4. REINFORCING STEEL: TYPICAL - ASTM A615, GRADE 60. FIELD BENT - ASTM A615, GRADE 40 WELDED

ROUGH CARPENTRY

- 1. ALL ROUGH CARPENTRY WORK SHALL BE EXECUTED IN CONFORMANCE WITH THE 9TH EDITION OF THE MASSACHUSETTS BUILDING CODE FOR ONE AND TWO FAMILY DWELLINGS (MBC 1&2) AND THE
- INTERNATIONAL RESIDENTIAL CODE FOR ONE AND TWO FAMILY DWELLINGS (IRC 1&2). 2. REFER TO THE MBC 1&2 AND IRC 1&2 FOR FRAMING COMPONENTS NOT SPECIFIED IN PLANS AND SECTIONS. NOTIFY THE ENGINEER OF ANY COMPONENT NOT DEFINED IN EITHER THE MBC 1&2 AND
- IRC 1&2 OR IN THESE DRAWINGS. 3. REFER TO THE IRC 1&2 FASTENER SCHEDULE FOR STRUCTURAL MEMBERS TABLE 602.3 FOR
- CONNECTION FASTENING NOT IDENTIFIED IN THESE PLANS OR DETAILS. 4. ENGINEER MAKES NO CLAIMS TOWARDS EXISTING CONDITIONS. 5. WHEN NOT OTHERWISE IDENTIFIED, ALL WOOD BEAMS, JOISTS, RAFTERS, HEADERS, STRINGERS, PLATES, AND SILLS SHALL BE SPRUCE PINE FIR #2 OR BETTER, WITH A MINIMUM Fb = 875 PSI (SINGLE
- USE) AND Fb = 1000 PSI (REPETITIVE USE), AND E SHALL BE 1,4000,000 PSI OR BETTER. 6. WOOD STUDS MAY BE EASTERN HEMLOCK, EASTERN SPRUCE, OR HEM-FIR, GRADED "STUD" GRADE,
- 7. LVL BEAMS, AS NOTED ON PLANS, SHALL HAVE A MINIMUM Fb = 3100 PSI, E = 2,000,000 PSI, AND Fv = 285 PSI. LVL BEAMS SHALL BE "VERSALAM" BY BOISE CASCADE. NO SUBSTITUTIONS WILL BE ACCEPTED, UNLESS THE ENGINEER SPECIFICALLY APPROVES ANOTHER PRODUCT SUBMITTED BY
- THE CONTRACTOR. 8. WOOD "I" BEAMS SHALL BE BY BOISE CASCADE. NO SUBSTITUTIONS WILL BE ACCEPTED, UNLESS THE ENGINEER SPECIFICALLY APPROVES ANOTHER PRODUCT SUBMITTED BY THE CONTRACTOR.
- MANUFACTURER'S RECOMMENDATIONS FOR BEARING, REINFORCING, CUTS, CANTILEVERS, FASTENING, ETC., SHALL BE STRICTLY ADHERED TO. 9. ENGINEERED WOOD POSTS (VERSA COLUMNS), AS NOTED ON PLANS, SHALL BE VERSA-LAM 1.7 2650. 10. PLYWOOD WALL SHEATHING, ROOF SHEATHING, AND SUBFLOORING SHALL BE APA GRADE,
- TRADEMARKED C-D INTERIOR WITH EXTERIOR GLUE. SUBFLOORING SHALL BE 3/4" THICK TONGUE AND GROOVE, AND SHALL BE GLUED TO FLOOR JOISTS WITH AN APPROVED ADHESIVE PRIOR TO NAILING. ROOF SHEATHING SHALL BE 5/8" THICK AND WALL SHEATHING SHALL BE 1/2" THICK. 11. ALL WOOD HAVING DIRECT CONTACT WITH CONCRETE OR MASONRY, AND WHEREVER WOOD IS
- WITHIN 8" OF FINISHED GRADE OR PART OF OPEN DECK CONSTRUCTION, SHALL BE PRESSURE 12. ALL METAL CONNECTORS INCLUDING JOIST AND BEAM HANGERS AND COLUMN CAP AND BASES
- SHALL BE BY SIMPSON STRONG-TIE CORP. THE CONTRACTOR SHALL STRICTLY ADHERE TO MANUFACTURER'S FASTENING REQUIREMENTS. CONTRACTOR TO VERIFY ALL CONNECTOR SIZES TO FRAMING ELEMENTS BEFORE ORDERING.
- 13. UNLESS DETAILED OR SPECIFIED OTHERWISE ON THE PLANS, HEADERS AND BEAMS SHALL BE
- SUPPORTED BY AT LEAST ONE JACK STUD AND ONE KING STUD. 14. FOR WOOD JOIST SPANS UP TO 14 FEET, PROVIDE A SINGLE ROW OF FULL DEPTH BLOCKING BETWEEN JOISTS AT MIDSPAN. FOR SPANS EXCEEDING 14 FEET, PROVIDE TWO ROWS OF FULL DEPTH BLOCKING BETWEEN JOISTS AT THIRD POINTS OF THE SPAN.
- 15. GABLE-END WALL STUDS IN CATHEDRAL, PARTIAL CATHEDRAL, OR HIGH CEILING SPACES SHALL SPAN UNINTERRUPTED FROM THE FLOOR PLATE TO THE UNDERSIDE OF THE ROOF RAFTERS. THEY SHOULD NOT BE INTERRUPTED BY ANY HORIZONTAL PLATES OR BEAMS, UNLESS NOTED
- OTHERWISE ON THE DRAWINGS. 16. MEMBERS WITHIN BUILT-UP BEAMS, WHETHER MADE OF SAWN OR ENGINEERED LUMBER, SHALL
- ONLY BE SPLICED OVER SUPPORTS. 17. PROVIDE SIMPSON H1 OR H2.5 HURRICANE TIES BETWEEN EACH RAFTER BOTTOM AND ITS BEARING
- 18. CONTRACTOR SHALL CAREFULLY COORDINATE THE WORK OF ALL TRADES TO MINIMIZE THE NEED FOR CUT, BORED OR NOTCHED IN FRAMING LUMBER. STRUCTURAL FLOOR MEMBERS SHALL NOT BE CUT, BORED OR NOTCHED IN EXCESS OF THE LIMITATIONS SPECIFIED IN THE BUILDING CODE
- WITHOUT WRITTEN APPROVAL FROM THE ENGINEER. 19. AT WOOD POSTS LANDING ON FLOOR DECK, PROVIDE SOLID VERTICAL WOOD BLOCKING WITHIN DECK SANDWICH TO LINK UPPER POST WITH LOWER SUPPORT. BLOCKING TO MATCH UPPER POST
- 20. SET LVL BEAMS THAT FRAME FLUSH WITH DIMENSIONED LUMBER JOISTS 3/8" BELOW THE TOP OF JOISTS TO ALLOW FOR JOIST SHRINKAGE. WHERE BEARING WALLS OR POSTS LAND ON THESE BEAMS, INFILL GAP WITH 3/8" PLYWOOD FOR SOLID BEARING.
- 21. BEAMS COMPRISED OF 3 LVLS OR MORE SHALL BE BOLTED TOGETHER WITH A MINIMUM OF 2-1/2". BOLTS AT 16" ON CENTER OR 3-1/4". DIAMETER SELF TAPPING LAG SCREWS AT 16" ON CENTER, ALTERNATING INSERTION SIDES, FOLLOW MANUF. SPECS, UNLESS NOTED OTHERWISE ON DRAWING.
- 22. IN ADDITION TO THE FLOOR JOIST SHOWN IN THE PLANS, CONTRACTOR SHALL INSTALL DOUBLE JOISTS UNDER ALL PARTITIONS WALLS RUNNING PARALLEL TO THE DIRECTION OF FRAMING.
- 23. MINIMUM BEAM BEARING TO BE 3 INCHES UNLESS NOTED OTHERWISE ON PLAN.

STRUCTURAL STEEL

- 1. STRUCTURAL STEEL WORK SHALL CONFORM TO THE AMERICAN INSTITUTE OF STEEL
- CONSTRUCTION: "SPECIFICATION FOR STRUCTURAL STEEL FOR BUILDINGS", LATEST EDITION. 2. STEEL WIDE FLANGE BEAMS SHALL CONFORM TO ASTM A992, WITH A MINIMUM YIELD STRENGTH OF 50 KSI. PLATES, ANGLES, CHANNELS, AND MISC. FABRICATED HARDWARE SHALL CONFORM TO ASTM A36, WITH A MINIMUM YIELD STRENGTH OF 36 KSI. RECTANGULAR STEEL TUBING SHALL CONFORM TO ASTM A500, GRADE B, WITH A MINIMUM YIELD STRENGTH OF 46 KSI.
- 3. ALL STEEL TO STEEL FIELD CONNECTIONS SHALL BE MADE BY HIGH STRENGTH BOLTING WITH ASTM A325 BOLTS OR WELDING WITH E70 XX ELECTRODES. STEEL TO CONCRETE AND STEEL TO WOOD
- FIELD CONNECTIONS MAY BE MADE WITH ASTM A 307 BOLTS. 4. STEEL SHALL BE SHOP-PAINTED WITH A MODIFIED ALKYD PRIMER UNLESS OTHERWISE NOTED.
- 5. NO CUTTING OF OR OPENINGS THROUGH STEEL WILL BE PERMITTED WITHOUT THE WRITTEN APPROVAL OF THE ENGINEER.
- 6. CONTRACTOR TO SUBMIT SHOP DRAWING TO ARCHITECT AND ENGINEER FOR APPROVAL.

LATERAL FRAMING NOTES

- 1. THE STRUCTURAL DESIGN OF THIS RESIDENCE WAS PERFORMED IN COMPLIANCE WITH THE INTERNATIONAL RESIDENTIAL CODE FOR ONE AND TWO FAMILY DWELLINGS. THE PRESCRIPTIVE REQUIREMENTS OF THIS CODE DO NOT APPLY PER SECTIONS 301.1.3 ALTERNATIVE PROVISIONS AND 301.1.3 ENGINEERED DESIGN.
- 2. FRAMING COMPONENTS AND FASTENERS AS IDENTIFIED IN THESE DRAWINGS AND NOTES ADEQUATELY RESIST THE LATERAL LOAD REQUIREMENTS AS DEFINED BY THE INTERNATIONAL
- RESIDENTIAL CODE FOR ONE AND TWO FAMILY DWELLINGS. 3. ALL EXTERIOR WALLS TO FOLLOW SHEARWALL SHEATHING CRITERIA.
 - SHEARWALLS CONSTRUCTION: SHEATHING TO BE ½" APA RATED
- SHEATHING TO BE ATTACHED TO THE WALL STUDS WITH 8dNAILS @ 4" OC AROUND EDGES & 8" OC IN FIELDS.
- HOLDDOWNS TO BE HDU5 BY SIMPSON AND SHALL BE ATTACHED TO A MIN OF 2-2x STUDS
- THREADED ROD TO BE 5/8"ø AND EPOXY SHALL BE SIMPSON SET-XP 6. ALL PLYWOOD SEAMS IN A SHEARWALL SHALL BE BLOCKED WITH DIMENSIONAL LUMBER OF THE
- SAME SIZE AS THE WALL STUDS. 7. REFER TO PLANS AND SECTIONS FOR STUD SIZES, STUDS SHALL BE SPACED AT 16 INCHES ON
- CENTER UNLESS NOTED OTHERWISE ON PLAN. 8. CARE SHOULD BE TAKEN TO ADJUST NAIL GUN PRESSURE SO AS TO NOT OVER DRIVE NAILS INTO PLYWOOD. NAIL HEADS SHOULD BE FLUSH WITH PLYWOOD FACE. OVER DRIVING NAILS GREATLY
- REDUCES THE EFFECTIVENESS OF THE SHEARWALL. 9. FOR FRAMING SIZES REFER TO FRAMING PLANS.

DESIGN LOADS PER MASSACHUSETTS STATE BUILDING CODE

LIVE LOADS

GROUND SNOW LOAD:	40 PSF
UNINHABITABLE ATTICS WITHOUT STORAGE:	10 PSF
UNINHABITABLE ATTICS WITH LIMITED STORAGE:	20 PSF
HABITABLE ATTICS AND SLEEPING AREAS:	30 PSF
ALL OTHER AREAS:	40 PSF

WIND LOADS

MASSACHUSETTS STATE BUILDING CODE: 100 MPH, EXPOSURE B

DEAD LOAD

WEIGHTS OF MATERIALS AND CONSTRUCTION

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DRAWING TITLE

GENERAL NOTES

Revision Schedule

Revision Description

SCALE

DATE

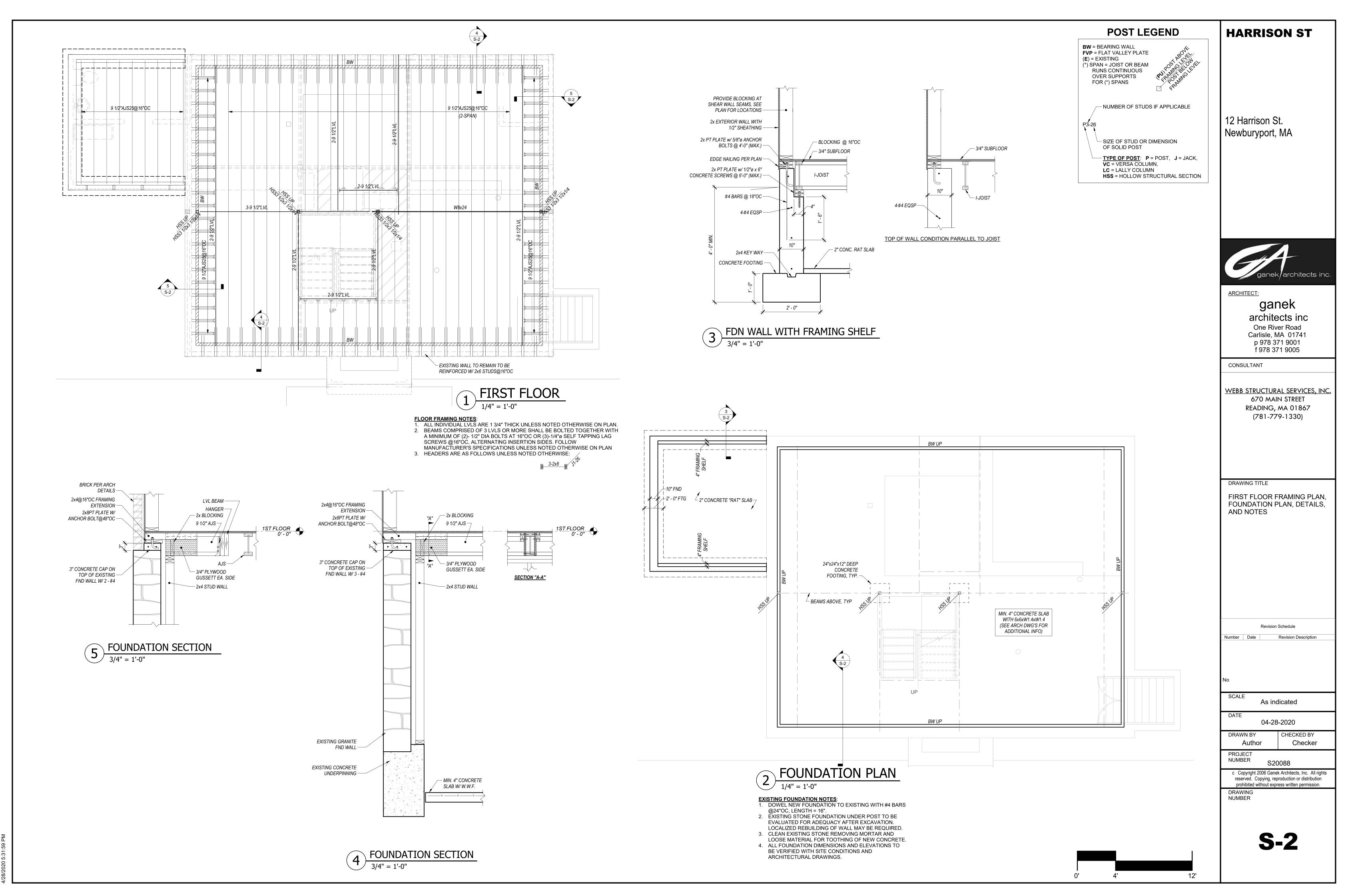
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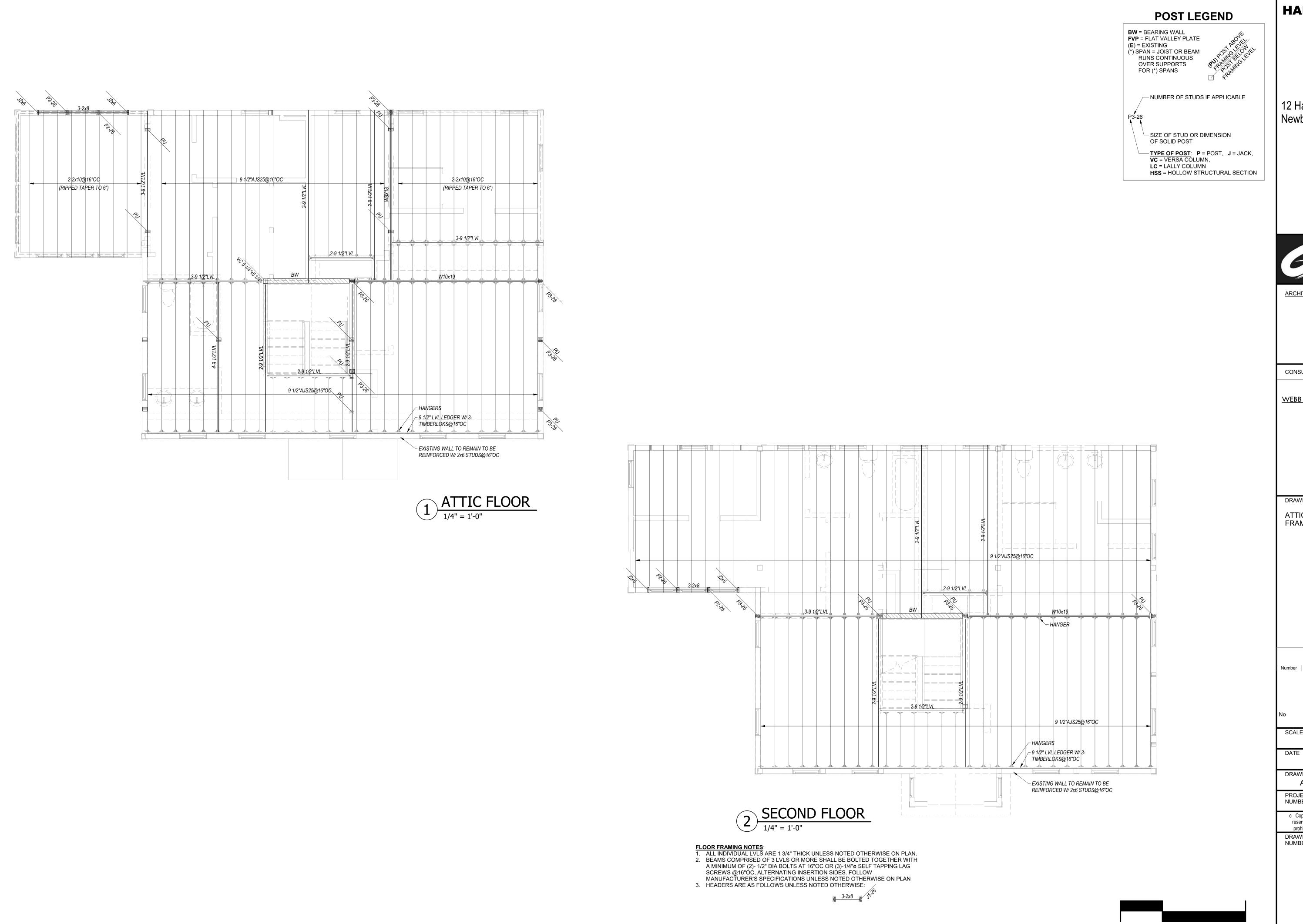
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DRAWING TITLE

ATTIC & SECOND FLOOR FRAMING PLANS

Revision Schedule

Revision Description Number Date

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04-28-2020

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S-3

______ 2-2x12 2x10@16"0C TRIMMER 2-14"LVL 2-2x12 FOLD LINE 2-2x12 TRIMMER TRIMMER

2-16"LVL RIDGE

2-14"LVL FOLD LINE

(2-SPAN)

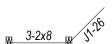


2x12@16"OC

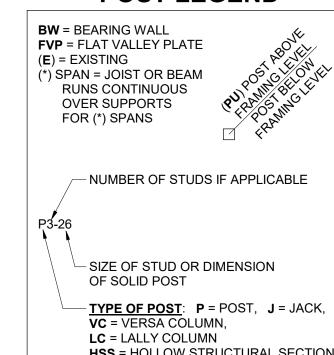
2-16"LVL RIDGE

- ROOF FRAMING NOTES: 1. ALL INDIVIDUAL LVLS ARE 1 3/4" THICK UNLESS NOTED OTHERWISE ON PLAN.
- 2. ALL RAFTER TO HIP OR VALLEY CONNECTION TO BE MADE W/ A MINIMUM OF 6-12d NAILS. 3. ALL CEILING TO RAFTER CONNECTIONS TO BE MADE W/ A MINIMUM OF 8-12d NAILS.
- 4. ALL RAFTER TO LVL RIDGE CONNECTIONS TO BE MADE WITH A MINIMUM OF 6-12d NAILS EQSP AND
- A SINGLE SIMPSON A34 FRAMING ANGLE UNLESS DETAILED OTHERWISE.

 5. ALL VALLEYS AND HIPS TO BE CONNECTED TO RIDGES WITH A MINIMUM OF 12 EQSP 16d NAILS AND
- A LS90 FRAMING ANGLE.
- 6. PROVIDE SIMPSON H2.5 HURRICANE TIES CONNECTING EACH RAFTER TO STRUCTURE BELOW. TIE TO BE PLACED OVER THE OUTSIDE WALL SHEATHING.
- 7. EXTEND PLYWOOD SHEATHING UNDER ENTIRE OVER FRAMED AREA.
- 8. BEAMS COMPRISED OF 3 LVLS OR MORE SHALL BE BOLTED TOGETHER WITH A MINIMUM OF (2)- 1/2"
- DIA BOLTS AT 16"OC OR (3)-1/4"ø SELF TAPPING LAG SCREWS @16"OC, ALTERNATING INSERTÌÓN
- SIDES. FOLLOW MANUFACTURER'S SPECIFICATIONS UNLESS NOTED OTHERWISE ON PLAN
 9. HEADERS ARE AS FOLLOWS UNLESS NOTED OTHERWISE:



POST LEGEND



HARRISON ST

TYPE OF POST: P = POST, J = JACK,
VC = VERSA COLUMN,
LC = LALLY COLUMN
HSS = HOLLOW STRUCTURAL SECTION

12 Harrison St. Newburyport, MA



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DRAWING TITLE

ROOF FRAMING PLAN

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